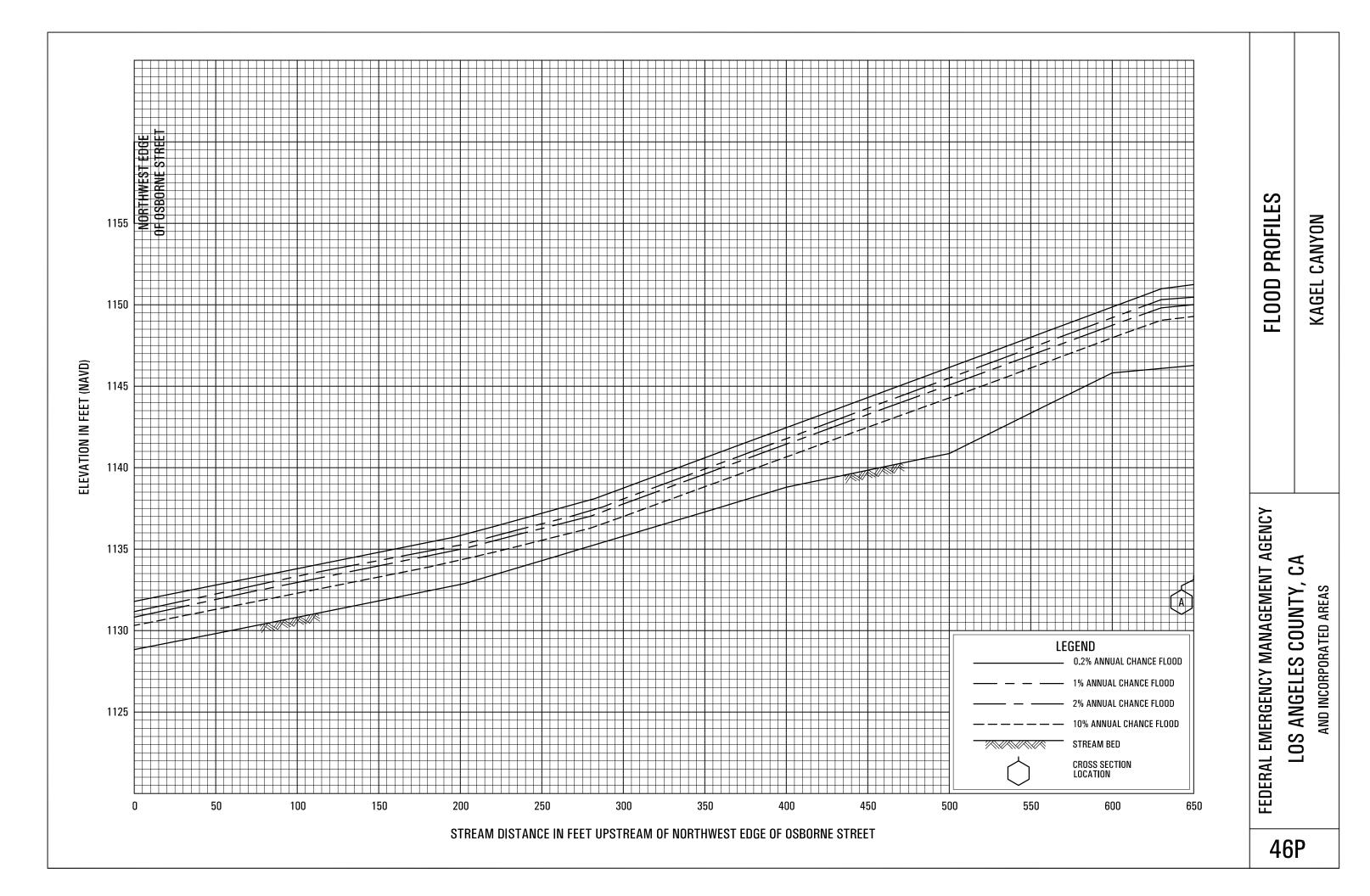
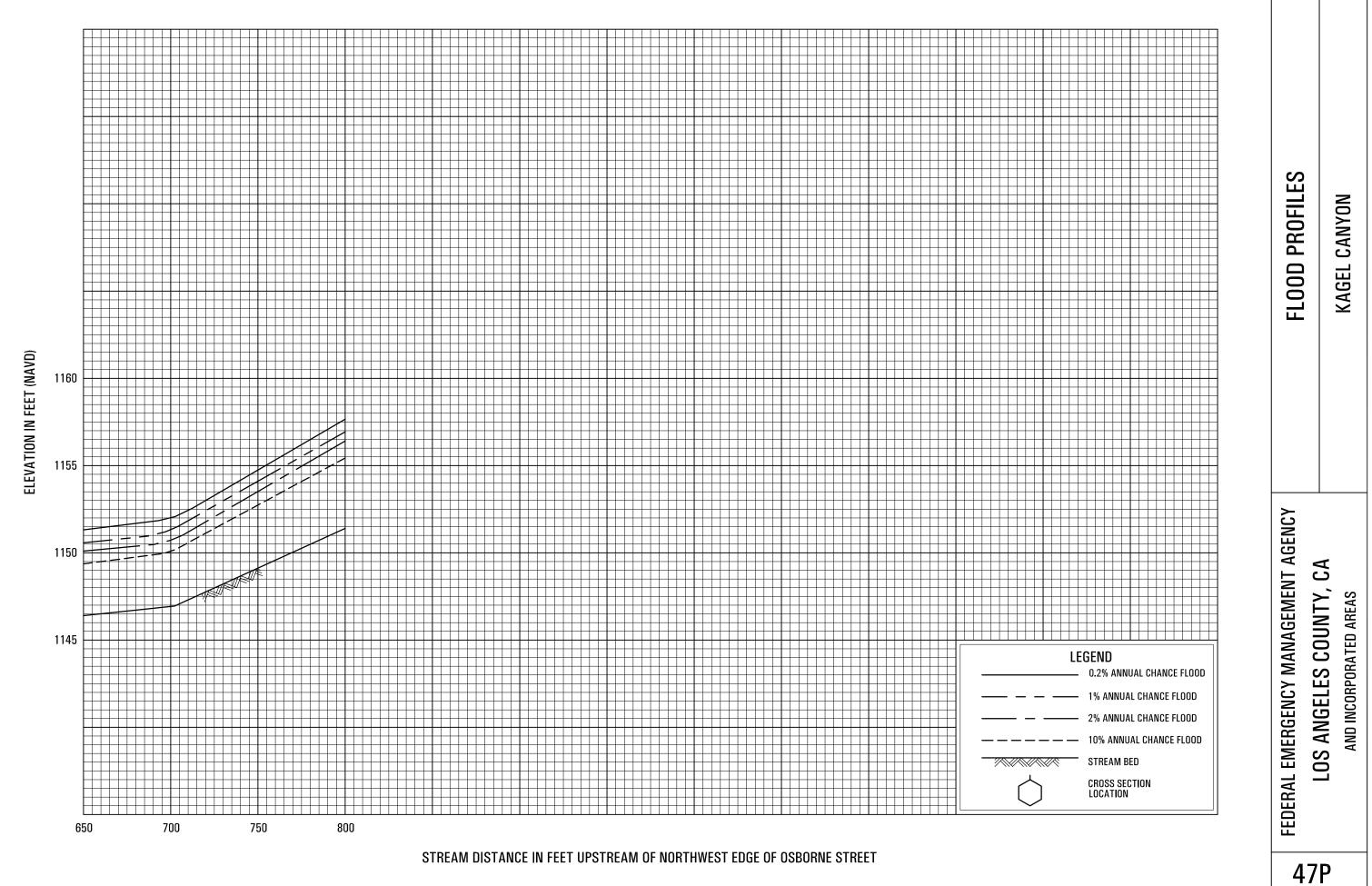
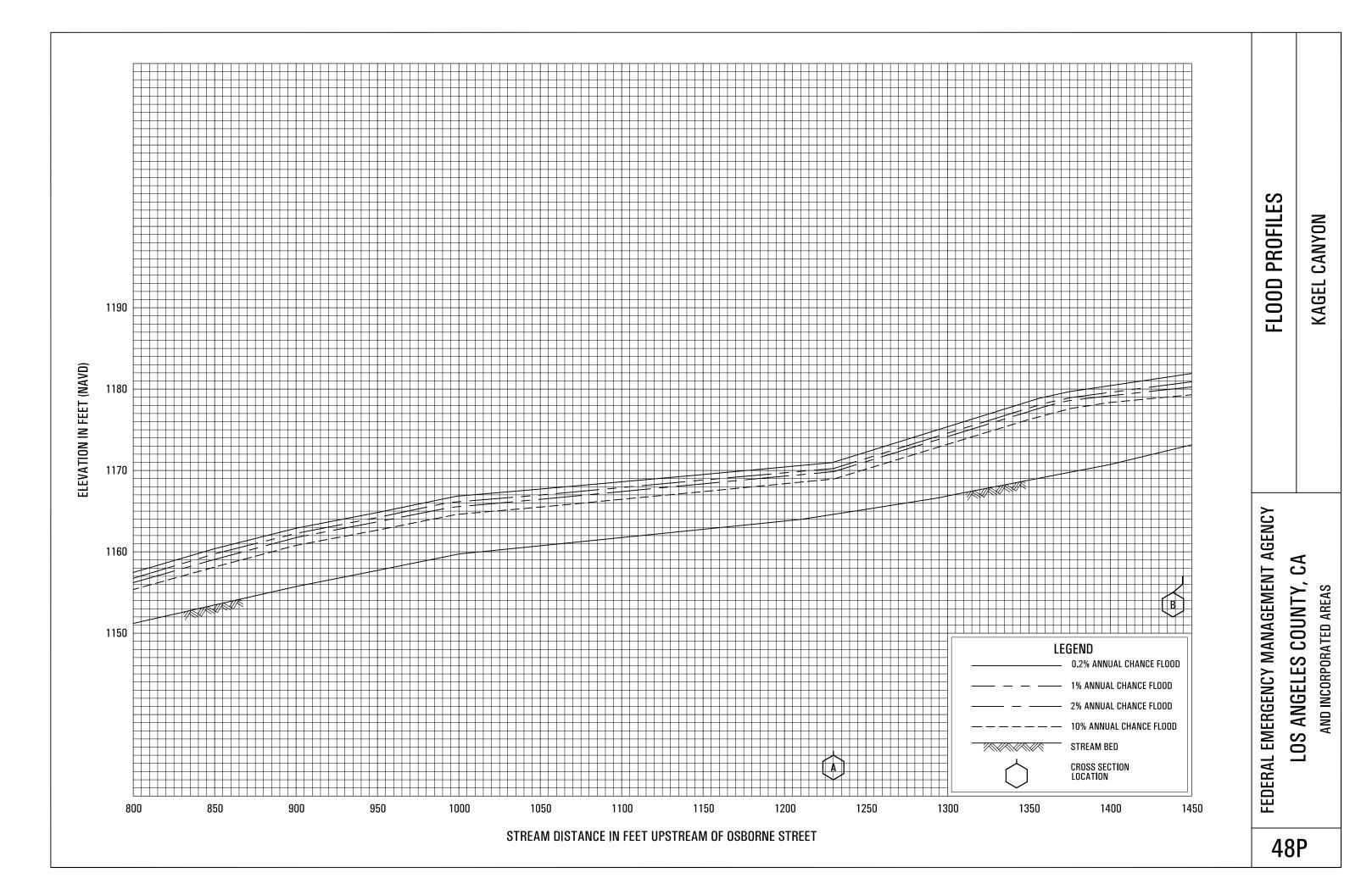
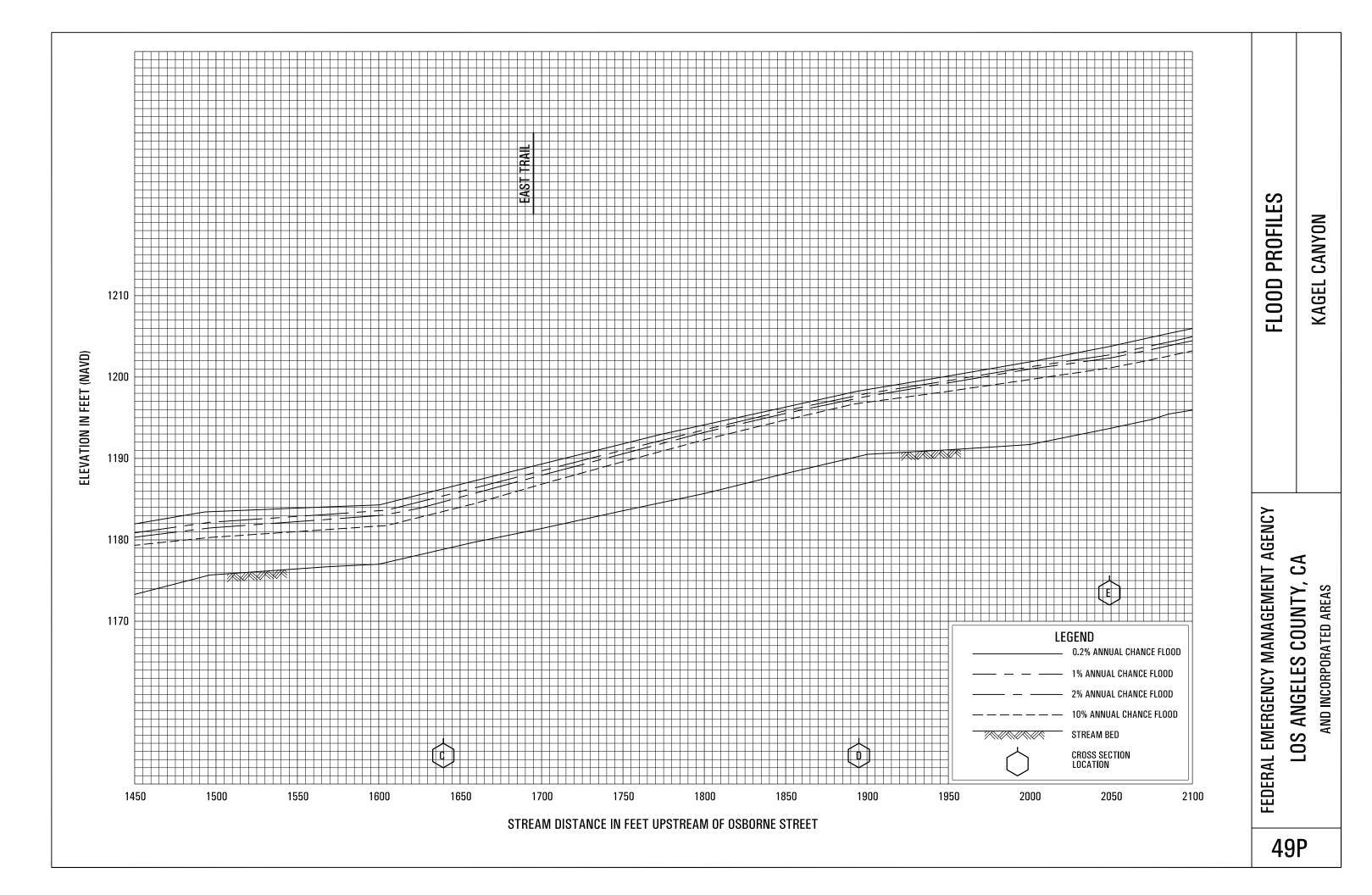
Appendix A –

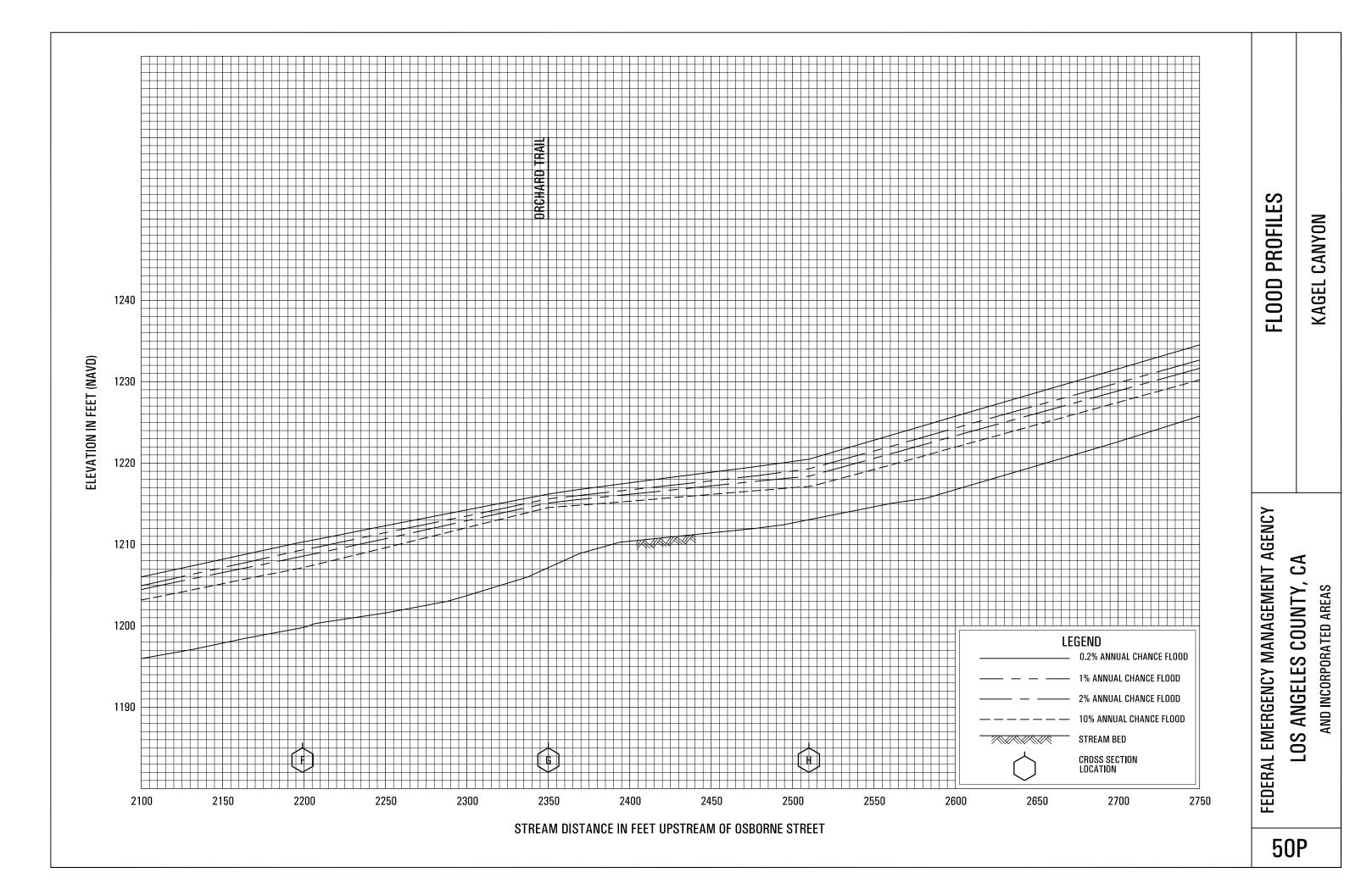
Effective FEMA Flood Profiles for Kagel Canyon (January 2016)

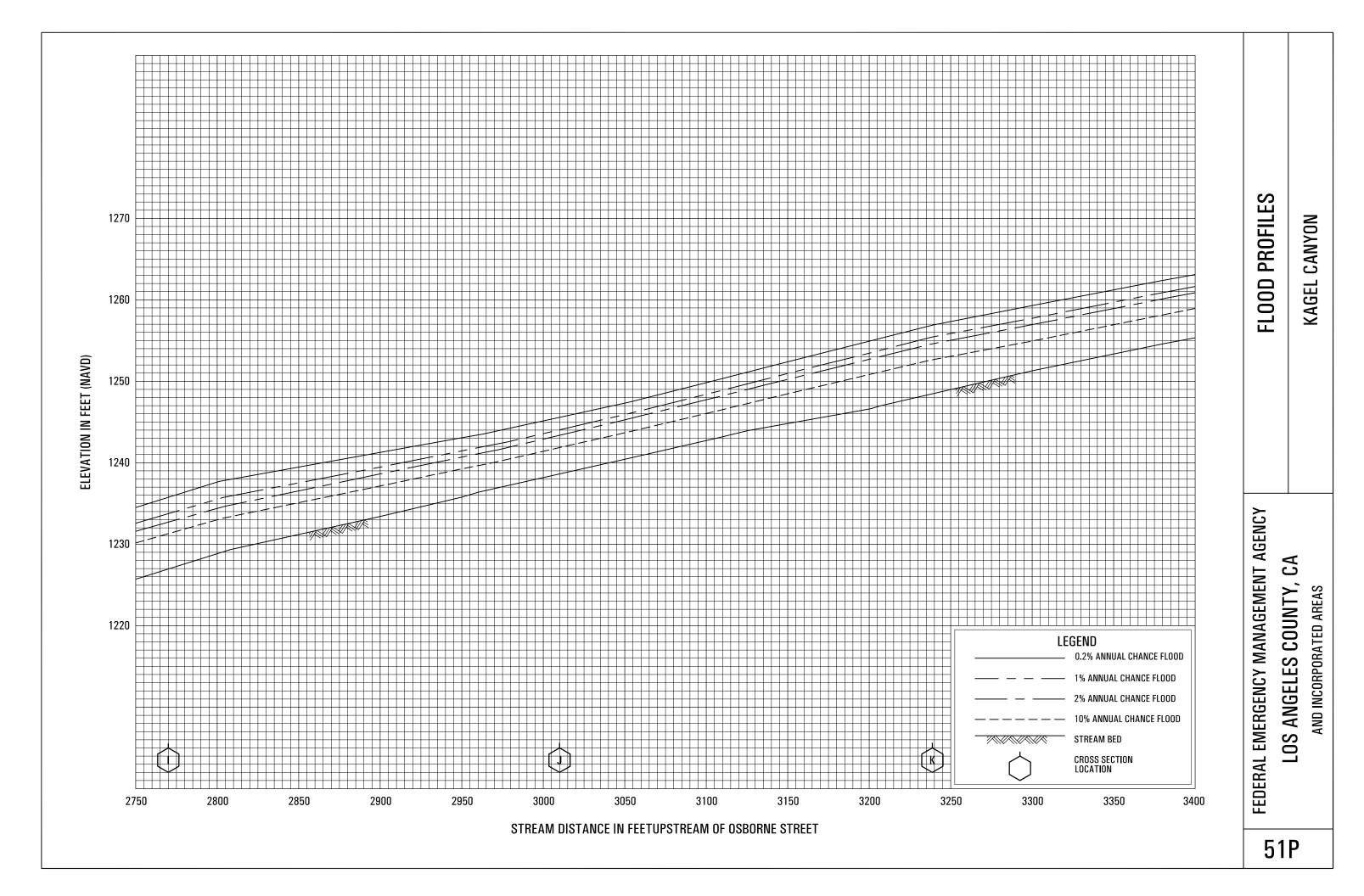


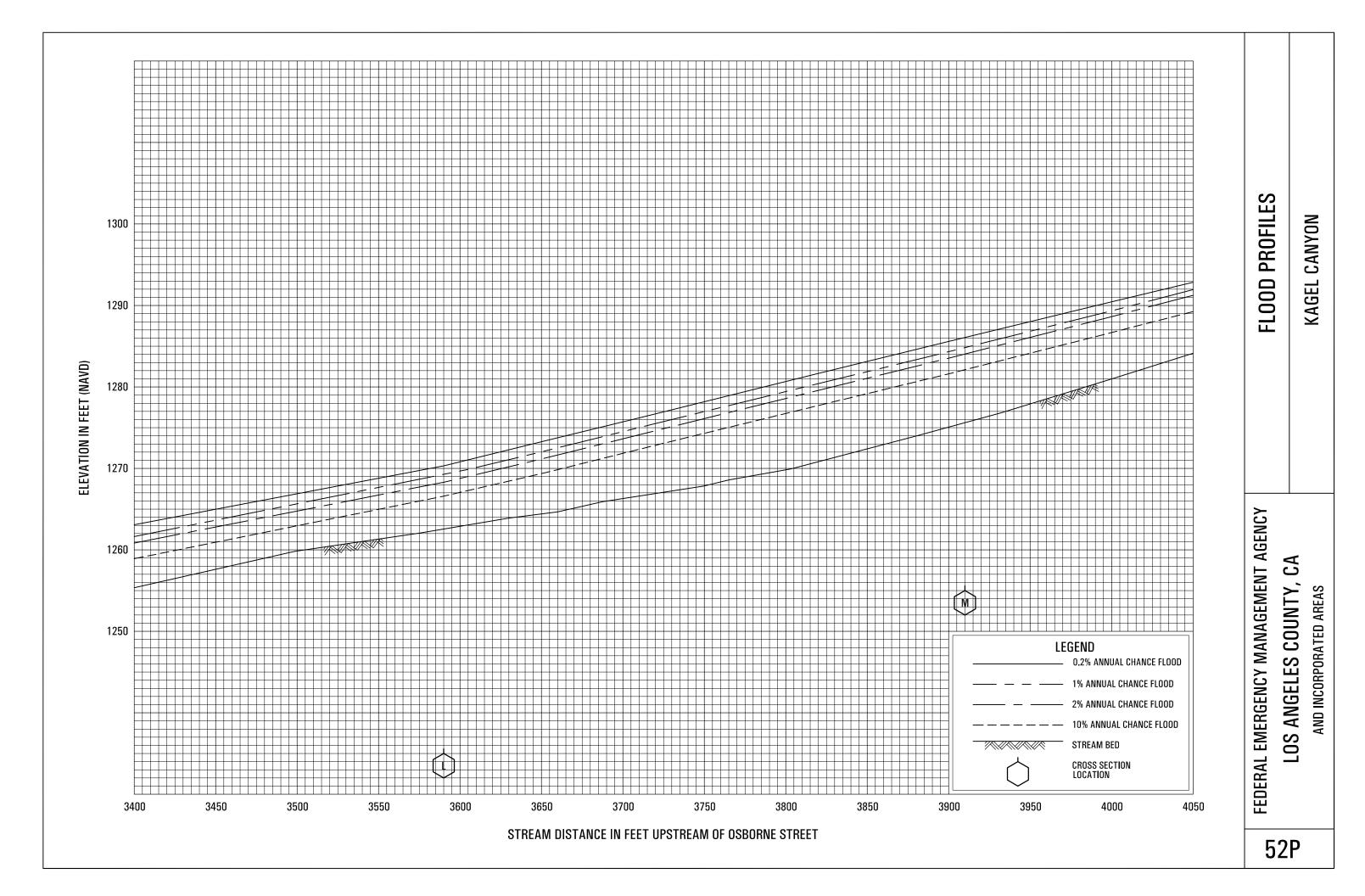


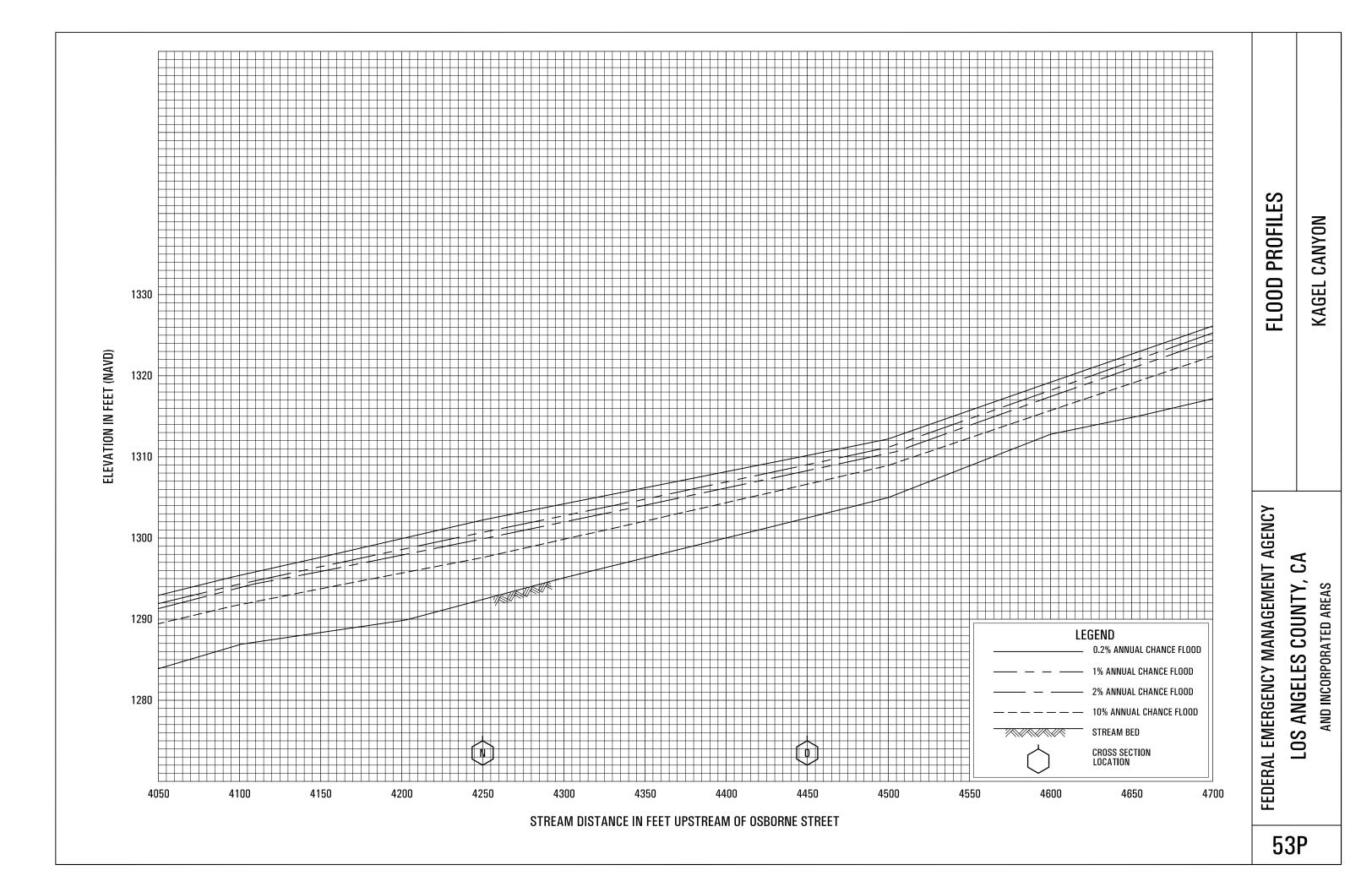


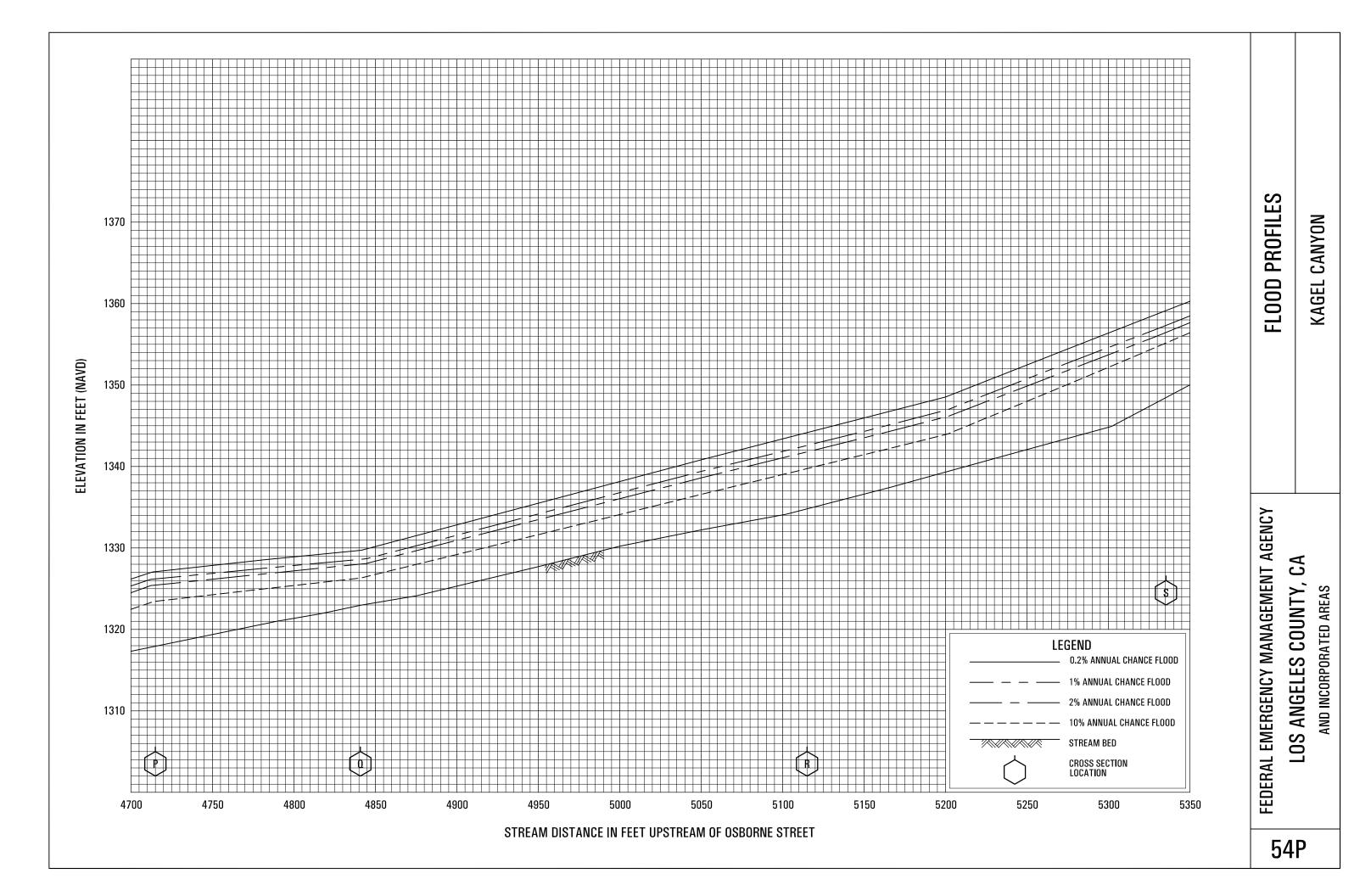


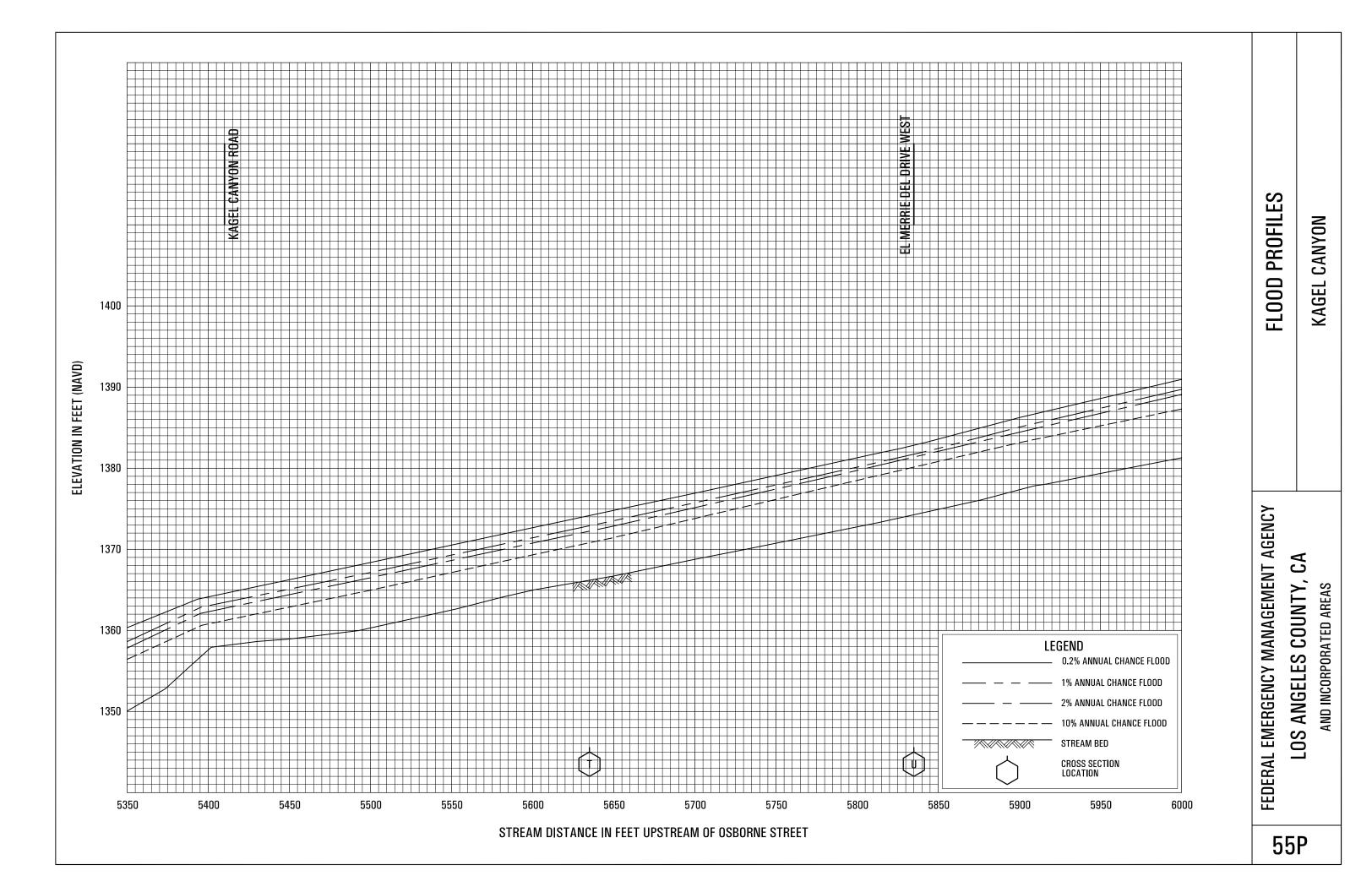


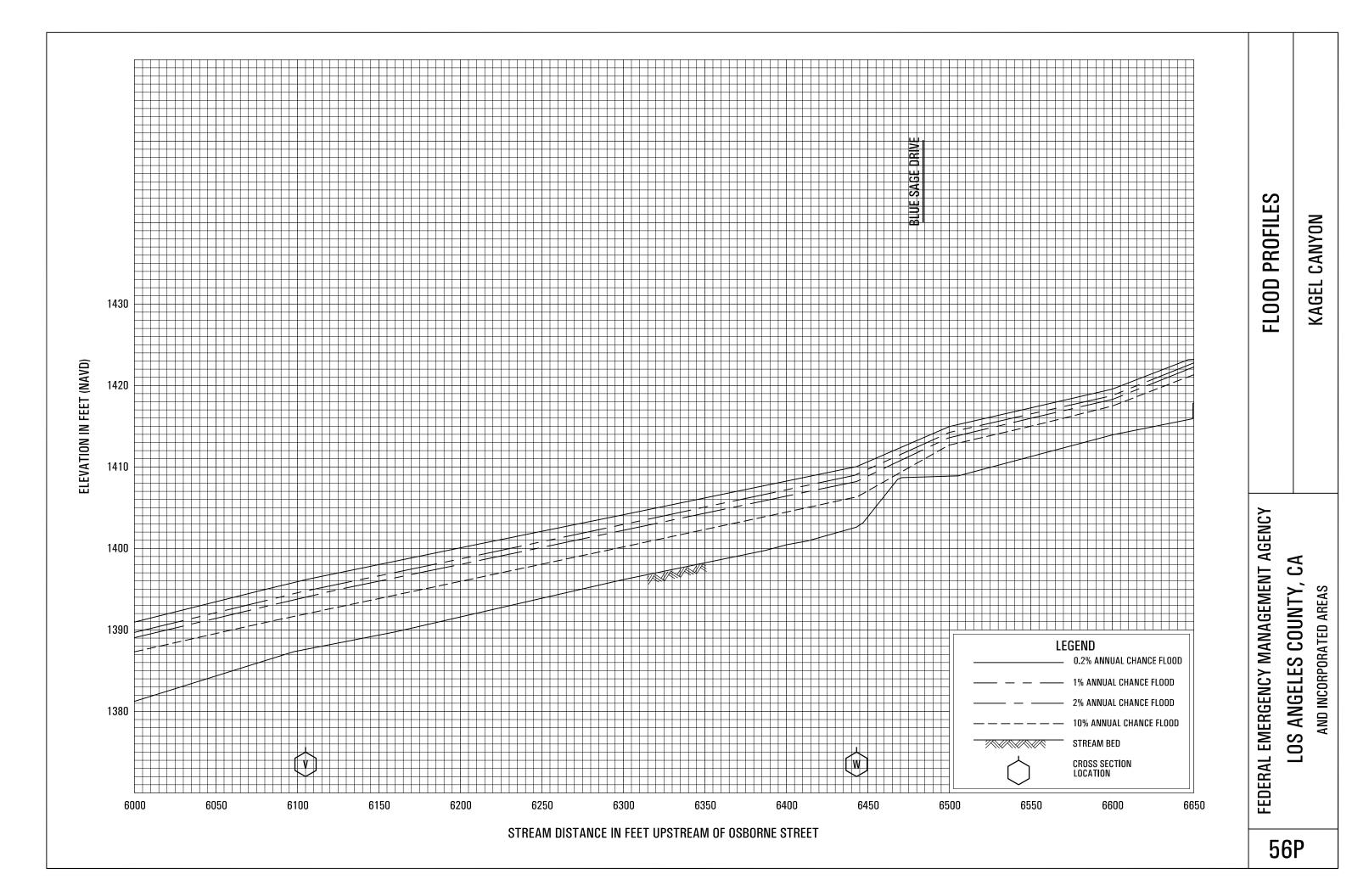


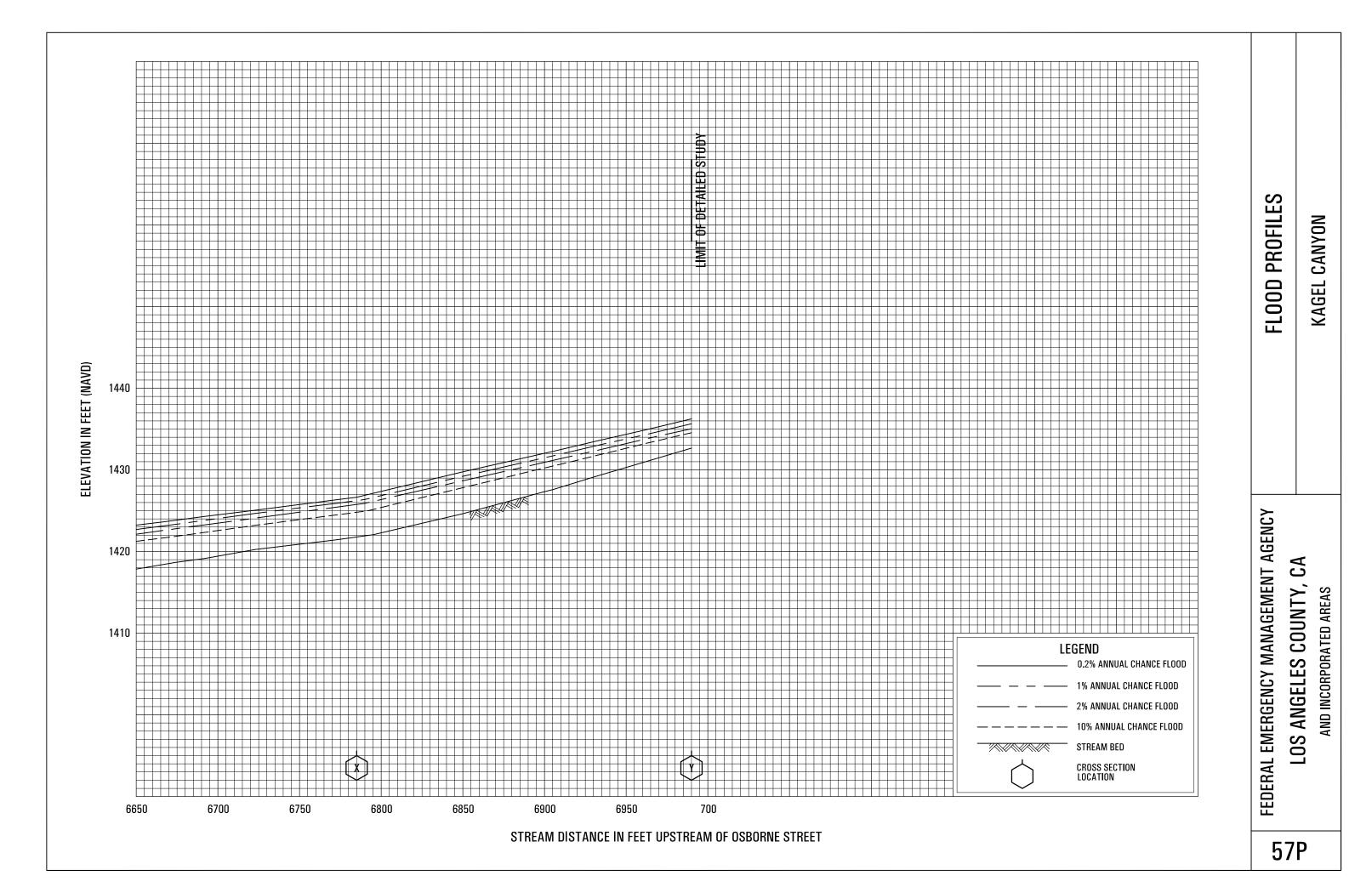












Appendix B – Field Investigation Site Photographs and Notes



Looking upstream at Str. 1 (Osborne St)



Looking downstream at Str. 1 (Osborne St)



Looking upstream at Kagel Canyon channel from Str. 1 (Osborne St.)



Looking downstream at Kagel Canyon channel from Str. 1 (Osborne St.)



Looking at confluence with Little Tujunga Wash



Looking upstream at Str. 2



Looking upstream at Kagel Canyon channel between Str. 1 and 2



Looking downstream at Kagel Canyon channel from Str. 2



Looking upstream at Kagel Canyon channel from Str. 2



Looking upstream at Str. 3



Looking downstream at Kagel Canyon channel from Str. 3



Looking upstream at Kagel Canyon channel from Str. 3



Looking downstream at Str. 3



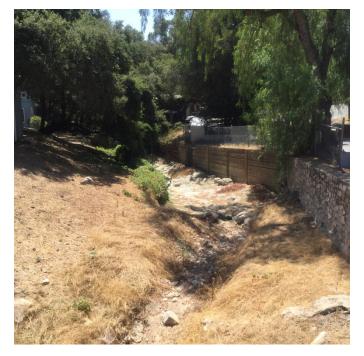
Looking downstream at Str. 4



Looking upstream at Str. 4



Looking upstream at Str. 5



Looking downstream at Kagel Canyon channel from Str. 6 (East Trl)



Looking upstream at Kagel Canyon channel and Str. 7 from Str. 6 (East Trl)



Looking upstream at Str. 6 (East Trl)



Looking downstream at Str. 6 (East Trl)



Looking upstream at Str. 7



Looking upstream at Str. 8



Looking upstream at Str. 9



Looking upstream at Str. 10



Looking downstream at Str. 10.5



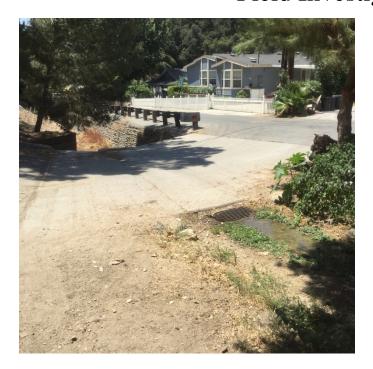
Looking upstream at Str. 11 (Orchard Trl, low water crossing)



Looking upstream at Kagel Canyon channel from Str. 10.5



Looking upstream at Kagel Canyon channel from Str. 11 (Orchard Trl)



Looking downstream at Str. 11 (Orchard Trl, low water crossing)



Looking upstream at Str. 12



Looking downstream at Str. 12



Looking upstream at Kagel Canyon channel from Str. 12



Looking upstream at Kagel Canyon channel between Str. 12 and 13



Looking upstream at Str. 13



Looking downstream at Str. 13



Looking downstream at Kagel Canyon channel from Str. 13



Looking upstream at Kagel Canyon channel from Str. 13



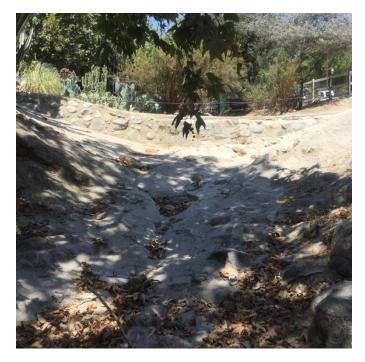
Looking downstream at Str. 14



Looking downstream at Kagel Canyon channel from Str. 14



Looking upstream at Kagel Canyon channel from Str. 14



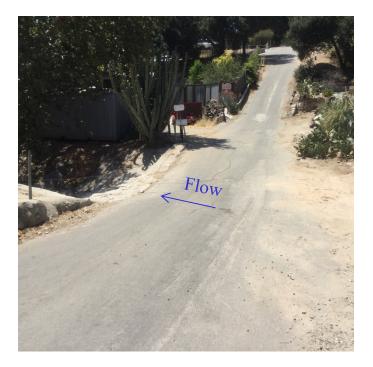
Looking upstream at Str. 15 (Blue Sage Dr, low water crossing)



Looking upstream at Kagel Canyon channel from Str. 15 (Blue Sage Dr)

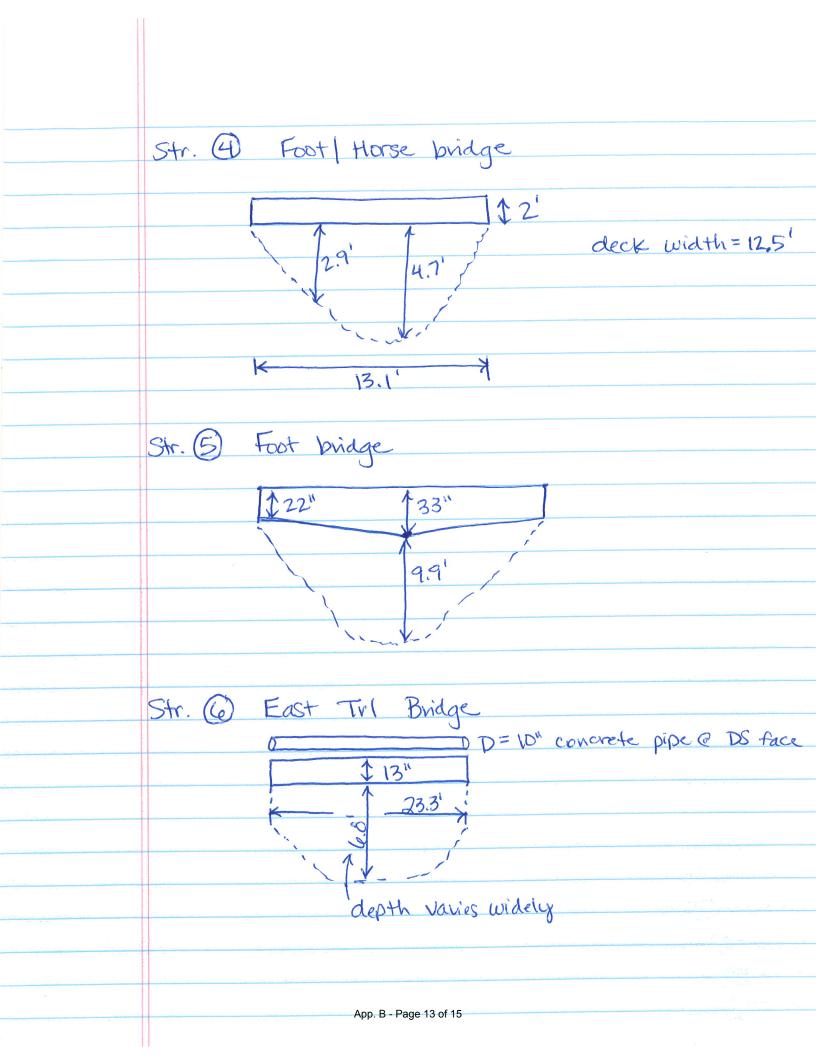


Looking downstream at Kagel Canyon channel from Str. 15 (Blue Sage Dr)

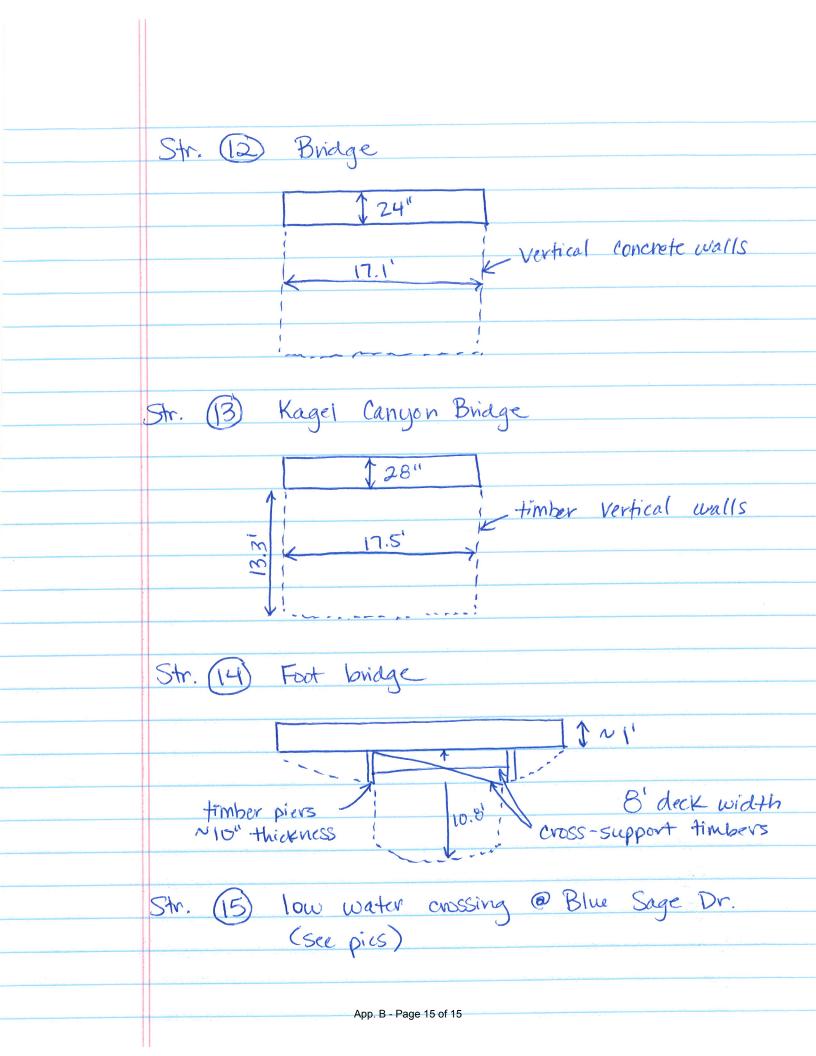


Looking at Str. 15 (Blue Sage Dr, low water crossing)

Kagel Canyon Field Investigation 8-4-14 Str. (D Double box @ Osborne St 15" round pier 15" to top of guard rail 13.1°CDS 10°CUS BODS 16' @ US 28" guard vail height 19.4' XK 19.1' X Str. 2 Foot bridge 1 15" 11.4' - timber vertical 16 channel banks m deck width = 5.5' Str. (3) Horse facility bridge \$28" Vertical Himber 10.3 × bank, no flow B' CUS face left of this K Point App. B - Page 12 of 15



Str. (7) Foot bridge (no rail) deck width = 3' deck thickness = 1' 1'21 E similar section for str. 7-10.5 Str. (B) Foot bridge (no rail) deck width = 3' deck thickness = 1' Str. 9 Foot bridge deck width = 3' deck thickness = 1.5' Str. (1) Foot bridge deck width = 3' deck thickness = 0.5 Str. (0.5 Foot bridge deck width = 3' deck thickness = 1 Str. (1) Low water crossing @ Orchard Trl US grate inlet w/ 12" dia boad way culvert fairly steep slope from road to DS channel App. B - Page 14 of 15



Appendix C -

Kagel Canyon Flood Hazard Study Hydrologic Analysis (Developed by LACDPW)

### KAGEL CANYON FLOOD HAZARD STUDY HYDROLOGIC ANALYSIS REPORT

Prepared By:



County of Los Angeles Department of Public Works Water Resources Division, Hydrology Section

June 2015

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2.6       Precipitation       9         2.7       Design Storm       12         2.8       Transform Function       13         2.9       Watershed Lag Time       14         2.10       Loss Method       15         2.11       Reach Routing       17         3.       RESULTS       18         3.1       HEC – HMS Results       18         3.2       Validation of HEC – HMS Results       18	2.4	Watershed Delineation	4
2.7       Design Storm       12         2.8       Transform Function       13         2.9       Watershed Lag Time       14         2.10       Loss Method       15         2.11       Reach Routing       17         3.       RESULTS       18         3.1       HEC – HMS Results       18         3.2       Validation of HEC – HMS Results       18	2.5	Land Use	7
2.8       Transform Function       13         2.9       Watershed Lag Time       14         2.10       Loss Method       15         2.11       Reach Routing       17         3.       RESULTS       18         3.1       HEC – HMS Results       18         3.2       Validation of HEC – HMS Results       18	2.6	Precipitation	9
2.9       Watershed Lag Time       14         2.10       Loss Method       15         2.11       Reach Routing       17         3.       RESULTS       18         3.1       HEC – HMS Results       18         3.2       Validation of HEC – HMS Results       18	2.7	Design Storm	12
2.10       Loss Method.       15         2.11       Reach Routing	2.8	Transform Function	13
2.11       Reach Routing       17         3.       RESULTS       18         3.1       HEC – HMS Results       18         3.2       Validation of HEC – HMS Results       18	2.9	Watershed Lag Time	14
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APPENDIX A – HEC-HMS RESULTS

APPENDIX B – HEC-SSP ANALYSIS

APPENDIX C – ELECTRONIC FILES

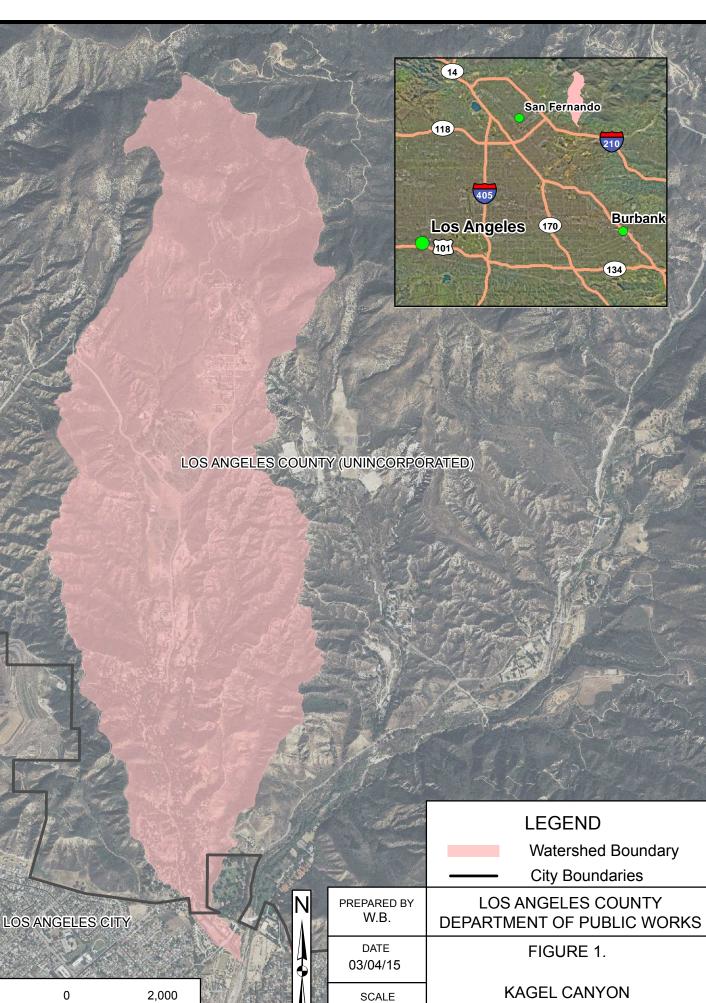
#### 1. **INTRODUCTION**

#### 1.1 Purpose

The purpose of this study is to develop a HEC-HMS model to simulate the 10year, 50-year, 100-year, and 500-year floods for the Kagel Canyon watershed. These peak flowrates will be used to evaluate the flood hazard area boundary for flood insurance purposes and will be part of a Letter of Map Revision application processed by the Department of Public Works. This report summarizes the methodology and hydrologic modeling approach used and the simulation results.

#### 1.2 Background

Kagel Canyon is located in the southwest part of Angeles National Forest east of San Fernando, just northeast of the intersection at Foothill Freeway (210) and the Ronald Reagan Freeway (118). Kagel Canyon is primarily an unincorporated community in Los Angeles County. The southern tip of the watershed is within the City of Los Angeles. Figure 1 shows the study's watershed location and boundary.



1 " = 2,000 '

Feet

2,000

WATERSHED BOUNDARY

#### 2. HYDROLOGIC ANALYSIS

#### 2.1 Watershed Area Characteristics

The Kagel Canyon watershed is approximately 2.31 square miles. The watershed is located within a rural area of the San Gabriel Mountains with some residential development. Kagel Canyon has an earthen-bottom channel that outlets to Little Tujunga Creek. The watershed has an elevation range from 3,380 feet to 1,120 feet.

#### 2.2 **Previous Study**

The original flood hazard mapping for the Kagel Canyon watershed was performed by the Los Angeles County Flood Control District (LACFCD) for FEMA, under Contract No. 27696. The LACFCD performed the hydrologic analysis to establish the peak discharge-frequency relationships for the flooding sources affecting the Los Angeles basin. Peak flow rates were computed using the Regional Runoff Frequency Equations developed by the LACFCD. These regional runoff frequency equations were developed through the multiple-linear regression analysis of the peak flow data of 48 gaging stations in Los Angeles County (FEMA 2008).

Peak discharges for Kagel Canyon are presented in the 2008 FEMA report and summarized in Table 1 below.

Flooding Source and	Drainage Area	Peak Discharges (cfs)						
Location	(sq. mi.)	10-yr	50-yr	100-yr	500-yr			
Kagel Canyon Channel (Cross Section A)	2.04	490	1,081	1,380	2,159			

#### Table 1. Peak Discharges for Kagel Canyon (2008 FEMA).

#### 2.3 Current Study Method

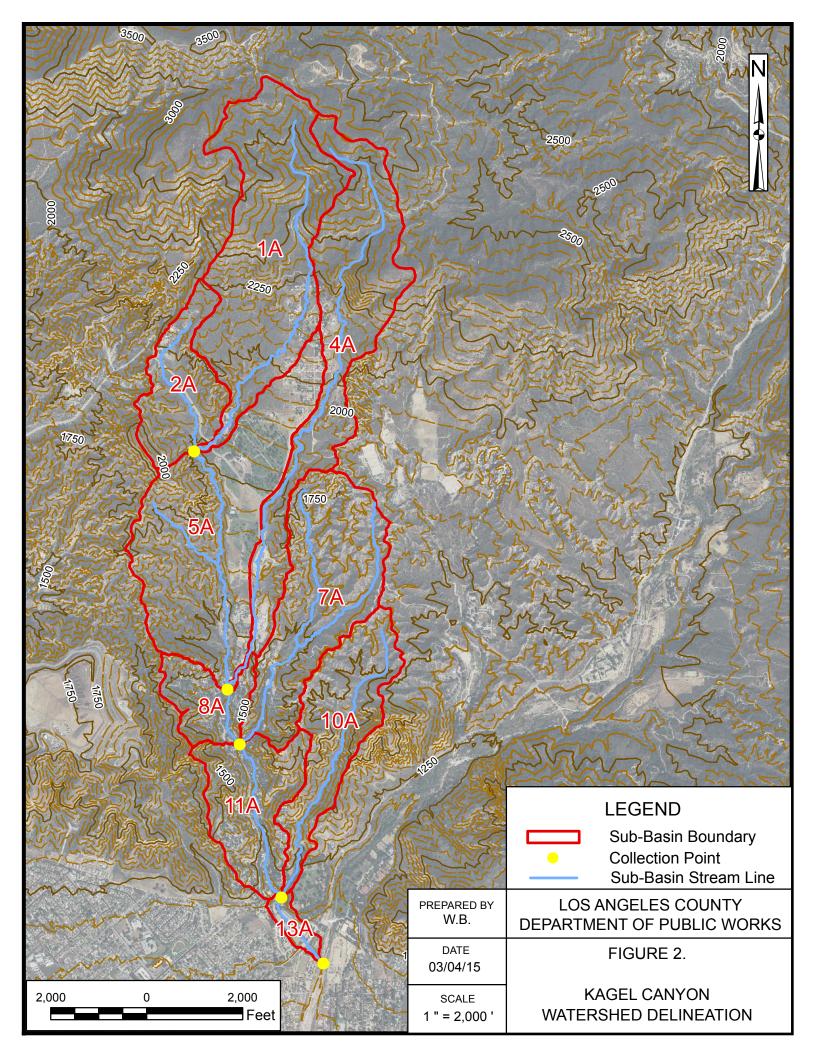
The hydrologic analysis prepared for the current study was developed using the U.S. Army Corps of Engineers (USACE) HEC-HMS, version 4.0. The hydrologic methods and procedures used in the hydrologic analysis are summarized in the following sections.

#### 2.4 Watershed Delineation

ArcGIS along with LiDAR 2-ft interval contours from 2006 were used to delineate the watershed into nine sub-basins as shown in Figure 2. The flow lengths and elevations for each sub-basin were also determined using ArcGIS.

The geographic information system (GIS) tools of WMS were utilized to compute areas, impervious values, and rainfall depths for each sub-basin. These watershed parameters were inputted into HEC-HMS.

The study watershed was divided into several sub-basins and concentration points based on the necessity for developing flow rates for the hydraulic modeling and analysis. A HEC-HMS model schematic diagram showing the sub-basins, reaches, and concentration points is presented in Figure 3.



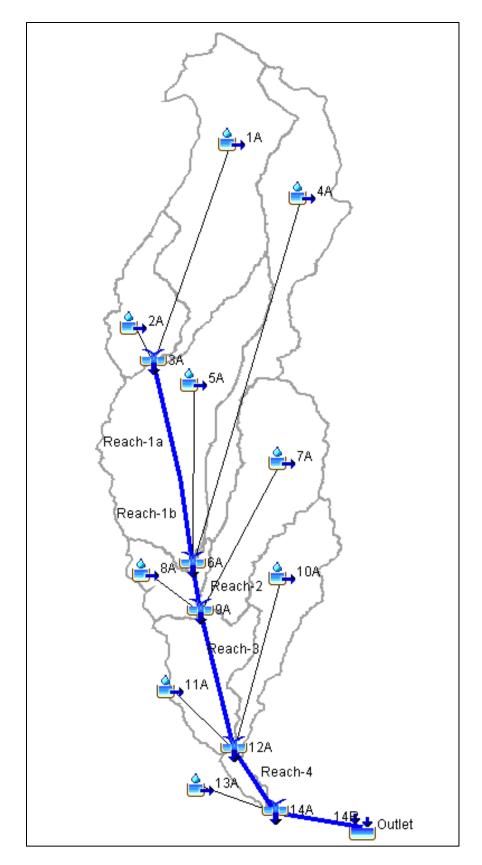
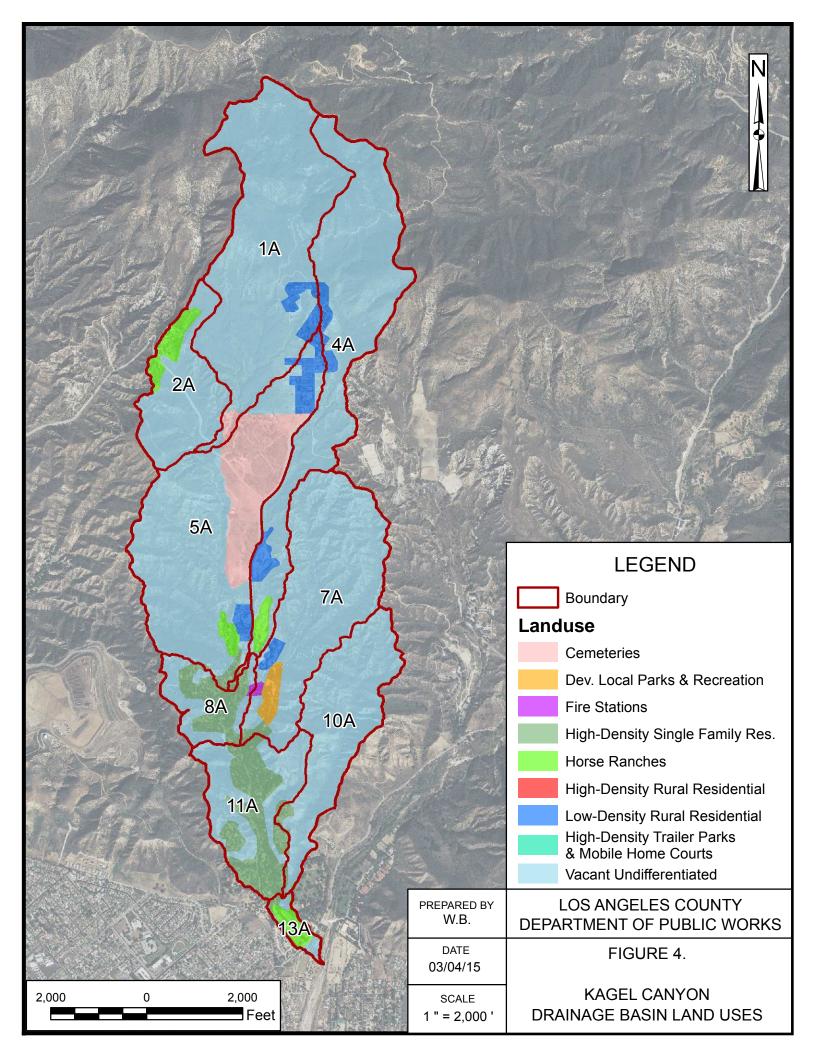


Figure 3. HEC-HMS Model Schematic.

### 2.5 Land Use

The study watershed area consisted of mainly undeveloped land and a small portion of residential land, verified by Google Map 2015 aerial imagery. The percent impervious value of each sub-basin was determined from land use information data compiled by the Southern California Association of Governments (SCAG) dated 2005. The area-weighted percent imperviousness of each sub-basin is listed in Table 2. A map showing the various land use types within the study area is provided in Figure 4.

Basin ID	Area (ac.)	Area (sq. mi.)	Impervious (%)
1A	297	0.465	1.5
2A	97	0.152	7.12
4A	248	0.388	2.86
5A	302	0.473	5.26
7A	207	0.324	2.7
8A	57	0.088	22.24
10A	137	0.214	2.46
11A	118	0.184	21.18
13A	15	0.024	31.97



### 2.6 Precipitation

Rainfall isohyetal maps published in the 2006 Los Angeles County Hydrology Manual were used to obtain precipitation values. The 50-year, 24-hour rainfall isohyetal map from LACDPW was used to determine the storm rainfall amount for the sub-basins. Figure 5 shows the 50-year, 24-hour rainfall isohyets within the study limits. Rainfall frequency multiplication factors of 0.714, 1.122, and 1.402 from the 2006 Los Angeles County Hydrology Manual were used to convert the 50-year, 24-hour rainfall depth values to 10-year, 100-year, and 500-year frequency depths, respectively. The partial duration rainfall depth values were computed using Equation 5.1.2 from the 2006 Hydrology Manual,

$$\frac{I_{t}}{I_{1440}} = \left(\frac{1440}{t}\right)^{0.47}$$

Where:

t = duration in minutes

 $I_t$  = rainfall intensity for the duration in inch/hour

 $I_{1440}$  = 24-hour rainfall intensity in inch/hour

Sub-basin rainfall depths for each frequency and duration are summarized in Table 3 through Table 6.

								<u>/·</u>
Basin ID	Duration							
Dasinid	5-min	15-min	60-min	2-hour	3-hour	6-hour	12-hour	24-hour
1A	0.24	0.44	0.91	1.32	1.63	2.36	3.41	4.92
2A	0.24	0.43	0.90	1.30	1.61	2.33	3.36	4.85
4A	0.24	0.43	0.89	1.29	1.59	2.30	3.32	4.80
5A	0.23	0.42	0.88	1.26	1.57	2.26	3.27	4.72
7A	0.23	0.41	0.86	1.24	1.53	2.21	3.19	4.61
8A	0.23	0.40	0.84	1.22	1.51	2.18	3.15	4.55
10A	0.22	0.40	0.84	1.21	1.49	2.16	3.12	4.50
11A	0.22	0.40	0.83	1.20	1.49	2.15	3.11	4.49
13A	0.22	0.40	0.83	1.20	1.49	2.15	3.11	4.49

Table 3. Kagel Canyon – 10-Year Frequency Rainfall Depths (in).

#### Table 4. Kagel Canyon – 50-Year Frequency Rainfall Depths (in).

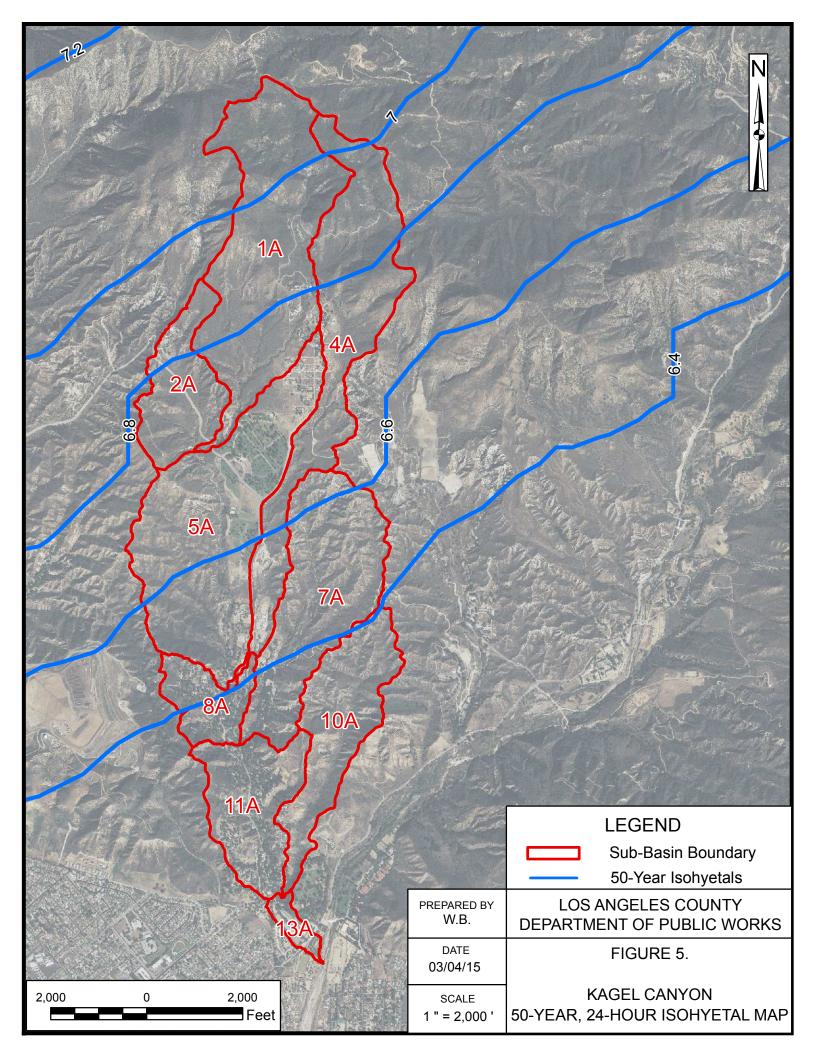
Basin ID	Duration							
Dasinid	5-min	15-min	60-min	2-hour	3-hour	6-hour	12-hour	24-hour
1A	0.34	0.61	1.28	1.84	2.29	3.30	4.76	6.88
2A	0.34	0.60	1.26	1.82	2.25	3.25	4.70	6.78
4A	0.33	0.60	1.25	1.80	2.23	3.22	4.65	6.72
5A	0.33	0.59	1.23	1.77	2.20	3.17	4.58	6.61
7A	0.32	0.57	1.20	1.73	2.14	3.09	4.47	6.45
8A	0.32	0.57	1.18	1.71	2.12	3.06	4.42	6.38
10A	0.31	0.56	1.17	1.69	2.10	3.03	4.37	6.31
11A	0.31	0.56	1.17	1.69	2.09	3.02	4.36	6.29
13A	0.31	0.56	1.17	1.69	2.09	3.02	4.36	6.29

Basin ID	Duration							
Dasinid	5-min	15-min	60-min	2-hour	3-hour	6-hour	12-hour	24-hour
1A	0.38	0.69	1.43	2.07	2.56	3.70	5.35	7.72
2A	0.38	0.68	1.41	2.04	2.53	3.65	5.27	7.61
4A	0.37	0.67	1.40	2.02	2.50	3.62	5.22	7.54
5A	0.37	0.66	1.38	1.99	2.46	3.56	5.14	7.42
7A	0.36	0.64	1.34	1.94	2.40	3.47	5.01	7.24
8A	0.36	0.64	1.33	1.92	2.38	3.43	4.95	7.15
10A	0.35	0.63	1.31	1.90	2.35	3.40	4.90	7.08
11A	0.35	0.63	1.31	1.89	2.35	3.39	4.89	7.06
13A	0.35	0.63	1.31	1.89	2.34	3.38	4.88	7.05

Table 5. Kagel Canyon – 100-Year Frequency Rainfall Depths (in).

 Table 6. Kagel Canyon – 500-Year Frequency Rainfall Depths (in).

Basin ID	Duration								
Dasinid	5-min	15-min	60-min	2-hour	3-hour	6-hour	12-hour	24-hour	
1A	0.48	0.86	1.79	2.59	3.21	4.63	6.68	9.65	
2A	0.47	0.85	1.76	2.55	3.16	4.56	6.59	9.51	
4A	0.47	0.84	1.75	2.52	3.13	4.52	6.52	9.42	
5A	0.46	0.83	1.72	2.48	3.08	4.45	6.42	9.27	
7A	0.45	0.81	1.68	2.42	3.01	4.34	6.27	9.05	
8A	0.44	0.80	1.66	2.40	2.97	4.29	6.19	8.94	
10A	0.44	0.79	1.64	2.37	2.94	4.24	6.12	8.84	
11A	0.44	0.78	1.63	2.36	2.93	4.23	6.10	8.81	
13A	0.44	0.78	1.63	2.36	2.93	4.23	6.10	8.81	



# 2.7 Design Storm

The HEC-HMS frequency storm option with peak intensity occurring at 50 percent of the storm duration was used in conjunction with partial duration rainfall depths for each sub-basin (Table 3 through Table 6). The 100-year temporal distribution is graphically presented in Figure 6.

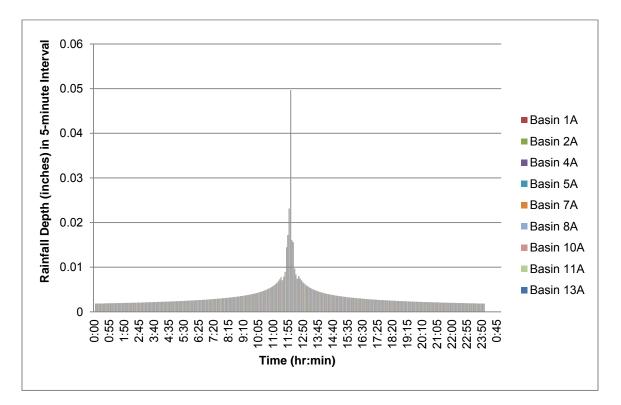


Figure 6. HEC-HMS Frequency Storm Temporal Distribution.

## 2.8 Transform Function

The surface runoff calculations were computed using the summation unit hydrograph (S-graph) method. The S-graph represents the response of a subbasin to a unit of precipitation. The mountain S-graph established by the USACE Los Angeles District is used in this study (Figure 7).

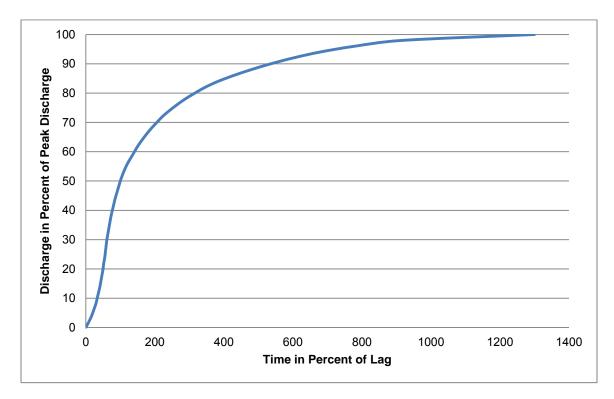


Figure 7. Mountain S-Graph.

### 2.9 Watershed Lag Time

The watershed lag time is defined as the time from the centroid of precipitation mass to the resulting hydrograph peak flow. The S-graph uses this parameter to transform the rainfall to runoff. The following equation from the USACE Los Angeles District (1962) was used to develop the sub-basins lag times:

$$L_{t} = 24 \times \bar{n} \times \left[\frac{L \times L_{c}}{\sqrt{s}}\right]^{0.38}$$

Where:

 $L_t$  = lag time in hours

n = basin roughness coefficient

L = longest watercourse length in miles

 $L_c$  = longest watercourse length, measured from the outlet upstream to a point opposite the drainage area centroid in miles

S = longest watercourse average slope in feet/mile

A basin roughness coefficient of 0.06 was used in the Kagel Canyon drainage area. This value was selected based on the roughness coefficient for rural surface cover provided in Table 7.3.1 from the 2006 Hydrology Manual. The watershed lag times for the sub-basins computed using the USACE lag equation are provided in Table 7.

		U				
Basin ID	Area (sq. mi.)	n	L (mi)	L <sub>c</sub> (mi)	S (ft/mi)	L <sub>t</sub> (hr)
1A	0.465	0.06	1.99	1.22	827.86	0.56
2A	0.152	0.06	0.87	0.31	657.17	0.26
4A	0.388	0.06	2.87	1.68	638.65	0.77
5A	0.473	0.06	1.59	0.66	461.30	0.46
7A	0.324	0.06	1.43	0.75	389.09	0.48
8A	0.088	0.06	0.54	0.25	596.66	0.20
10A	0.214	0.06	1.45	0.86	361.59	0.51
11A	0.184	0.06	0.81	0.41	579.63	0.28
13A	0.024	0.06	0.38	0.20	313.99	0.18

 Table 7. Kagel Canyon Sub-basin Lag Time.

### 2.10 Loss Method

The initial loss and constant loss rate method in HEC-HMS was used for this study to simulate watershed losses. The initial loss specifies the amount of precipitation that falls on the watershed without producing runoff and the constant rate defines the infiltration rate that occurs after the initial loss is satisfied. These losses were determined based on the watershed's physical properties: soil, land use, and antecedent conditions. GIS shapefiles of soil types from the Natural Resources Conservation Service (NRCS) for the Angeles National Forest Area and Los Angeles County, West San Fernando Valley Area (NRCS 2004) were used in determining the hydrologic soil groups (A, B, C, or D) in the project area (Figure 8).

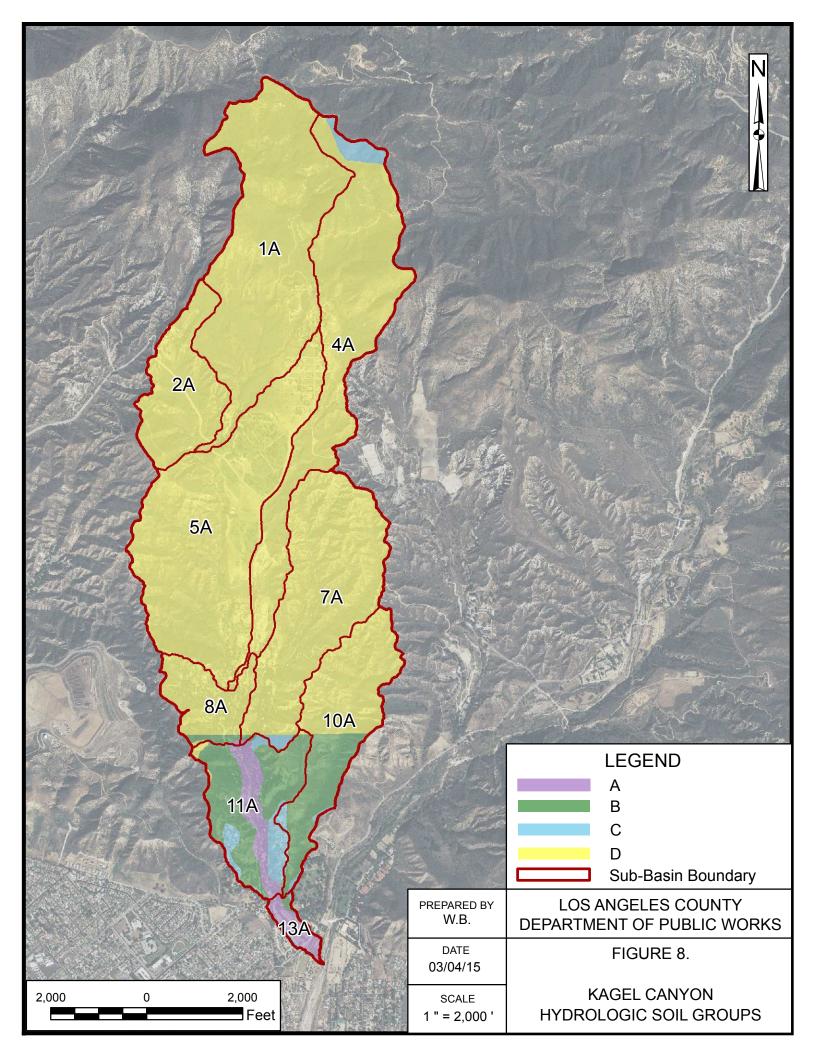
Ranges of infiltration rates for each NRCS soil group were taken from the HMS Technical Reference Manual (Chapter 5, Table 11). The maximum value of the given ranges for each hydrologic soil group was used as shown in Table 8 below. Area-weighted averages were determined for sub-basins with more than one hydrologic soil group. The hydrologic soil groups and their corresponding constant loss rates for each sub-basin are summarized in Table 9. Initial loss was considered to be zero for all sub-basins since it was assumed that the watershed is fully saturated.

NRCS Soil Group	Constant Loss Rate (in/hr)
A	0.45
В	0.30
С	0.15
D	0.05

Table 8. Constant Loss Rate.

Table 9. Kagel Canyon	Sub-basin Soil Grou	ips and Constant Loss Rates.
-----------------------	---------------------	------------------------------

Basin ID	NRCS Soil Group	Constant Loss Rate (in/hr)			
1A	D	0.05			
2A	D	0.05			
4A	C, D	0.0543			
5A	D	0.05			
7A	A, B, C, D	0.0529			
8A	A, B, D	0.0701			
10A	B, C, D	0.1432			
11A	A, B, C, D	0.3036			
13A	A, B, C	0.4002			



### 2.11 Reach Routing

The Muskingum-Cunge method was used to route flows from one concentration point to the next. Since no single standard cross-section shape can represent the channel geometry for the entire Kagel Canyon watershed, the 8-point cross-section configuration was used for each routing reach. A representative cross section for each routing reach was defined using 8 pairs of x, y (distance, elevation) values. Reach lengths, roughness coefficients, and channel slopes were also determined and summarized in Table 10. Reach 1 was divided into two segments, 1a (upstream) and 1b (downstream), because the contour lines showed significant changes in channel shape from upstream to downstream.

Reach	Length (ft)	Slope (ft/ft)	Manning's n	Left Manning's n	Right Manning's n
1a	3,640	0.057	0.060	0.060	0.060
1b	2,281	0.046	0.060	0.060	0.060
2	1,261	0.051	0.025	0.043	0.043
3	3,613	0.045	0.045	0.045	0.045
4	1,755	0.040	0.050	0.050	0.050

Table 10. Muskingum-Cunge Parameters.

# 3. **RESULTS**

# 3.1 HEC – HMS Results

The HEC-HMS model simulated the 10-year, 50-year, 100-year, and 500-year peak flow rates at five concentration points within the watershed. The results are summarized in Table 11. The computed peak flow rates at the watershed outlet for the 10-year flood is 1,067 cfs, for the 50-year flood is 1,572 cfs, for the 100-year flood is 1,790 cfs, and for the 500-year flood is 2,270 cfs. See Appendix A for HEC-HMS summary output.

Table TT. Feak Discharge Results Summary.								
Concentration	Drainage Area		Peak Discharge (cfs)					
Point	(sq. mi.)	10-Year	50-Year	100-Year	500-Year			
3A	0.62	341	485	547	688			
6A	1.48	729	1,052	1,191	1,505			
9A	1.89	924	1,354	1,536	1,941			
12A	2.29	1,069	1,568	1,779	2,265			
14A	2.31	1,067	1,572	1,790	2,270			

Table 11. Peak Discharge Results Summary.

# 3.2 Validation of HEC – HMS Results

Kagel Canyon is an un-gaged watershed; therefore, no calibration was performed. In order to validate the results of HEC-HMS model, four USGS gaging stations with adequate streamflow data and similar hydrologic characteristics to the Kagel Canyon watershed were selected. Table 12 lists the selected gaging stations.

A flood flow frequency analysis, based on Bulletin 17B, was conducted for the gaging stations. Table 13 summarizes the results. Appendix B includes the full results for the flood flow frequency analysis.

Table 12. USGS Stream Gages in the vicinity of Kagel Canyon.							
USGS Stream Gage Name	Gage Station	Drainage Area (sq. mi.)	Period of Record	Years of Record			
Kagel Canyon	-	2.31	-	-			
Rogers C NR Azusa CA	11084000	6.64	1918-1962	44			
Little Dalton C NR Glendora CA	11086500	2.72	1939-1971	32			
Little Tujunga C NR San Fernando CA	11096500	21.1	1929-1973	44			
Arroyo Seco NR Pasadena CA	11098000	16.0	1914-2013	99			

 Table 12. USGS Stream Gages in the Vicinity of Kagel Canyon.

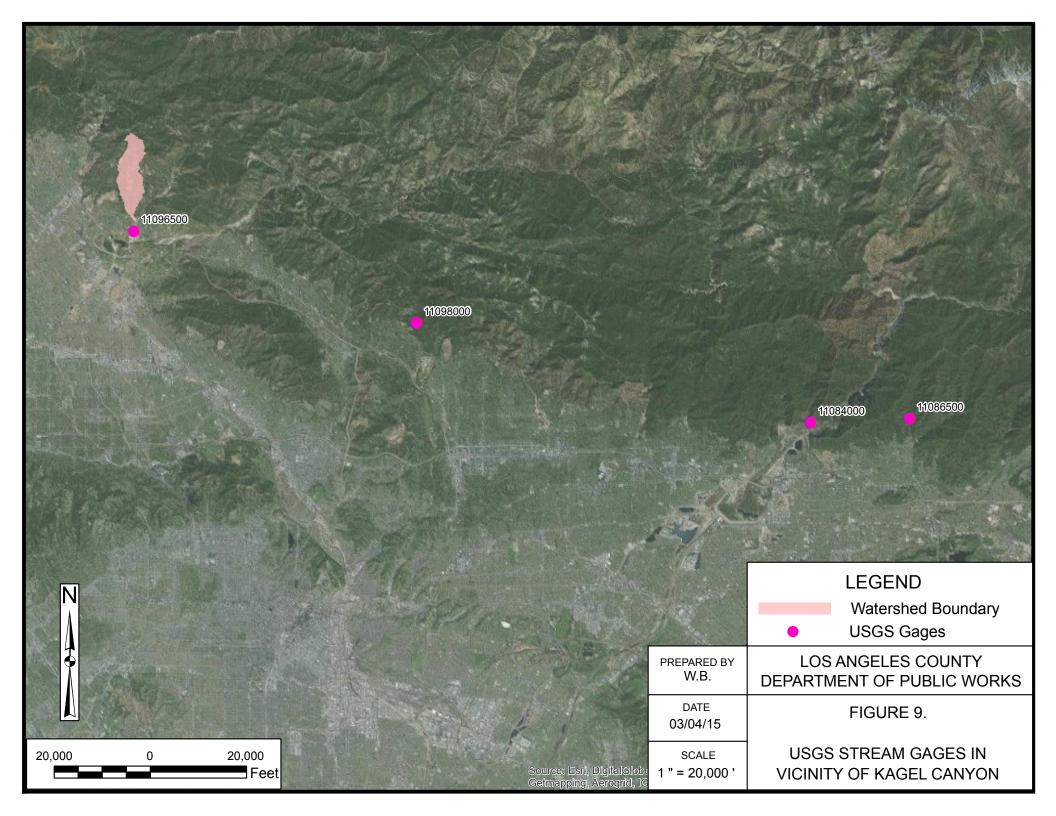
Case Station		Peak Dis	charge (cfs)				
Gage Station	10-Year	50-Year	100-Year	500-Year			
11084000	1,158	2,921	3,974	7,195			
11086500	328	999	1,458	3,062			
11096500	2,496	7,211	10,156	19,338			
11098000	2,864	6,971	9,347	16,384			

Table 13. Peak Discharge	for	Stream	Gages.
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To validate the HEC-HMS simulation results, the unit peak flow rate per acre corresponding to several recurrence intervals for Kagel Canyon and gaging stations were computed. Table 14 shows the comparisons. The comparison shows that the HEC-HMS simulates extreme events, such as the 100-year and 500-year frequency, more closely to the results from the flood flow frequency analysis.

Cogo Station	Peak Discharge per Area (cfs/ac.)						
Gage Station	10-Year	50-Year	100-Year	500-Year			
Kagel Canyon	0.722	1.063	1.210	1.535			
11084000	0.272	0.687	0.935	1.693			
11086500	0.189	0.574	0.837	1.759			
11096500	0.185	0.534	0.752	1.432			
11098000	0.280	0.681	0.913	1.600			

Table 14. Peak Discharge per Area.



# 4. **REFERENCES**

County of Los Angeles Department of Public Works. <u>Hydrology Manual</u>. January 2006.

FEMA (Federal Emergency Management Agency). <u>Flood Insurance Study, Los Angeles</u> <u>County, California and Incorporated Areas</u>. September 26, 2008.

NRCS (Natural Resources Conservation Service). 2004. <u>Soil Survey Geographic</u> (SSURGO) database for Angeles National Forest Area, California and Los Angeles County, California, West San Fernando Valley Area. http://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx

SCAG (Southern California Association of Governments). <u>Land Use Database</u>. Los Angeles, California. 2005

U.S. Army Corps of Engineers, Hydrologic Engineering Center. <u>Hydrologic Modeling</u> <u>System HEC-HMS, Technical Reference Manual.</u> March 2000.

U.S. Army Corps of Engineers, Hydrologic Engineering Center. <u>Hydrologic Modeling</u> <u>System HEC-HMS, User's Manual.</u> December 2013.

U.S. Army Corps of Engineers, Hydrologic Engineering Center. <u>Statistical Software</u> <u>Package HEC-SSP, User's Manual, Version 2.0.</u> October 2010.

U.S. Army Corps of Engineers, Los Angeles District. Los Angeles County Drainage Area Final Feasibility Interim Report, Part 1 Hydrology Technical Report, Base Conditions. December 1991.

U.S. Interagency Advisory Committee on Water Data. <u>Guidelines for Determining Flood</u> <u>Flow Frequency, Bulletin 17B of the Hydrology Subcommittee</u>. March 1982.

# APPENDIX A

HEC-HMS OUTPUT

	Global Summar	/ Results for Run '	"10yr w/ max cl"
--	---------------	---------------------	------------------

- • •

	Project: Kagel_Fi	nal Simulation R	un: 10yr w/ max d	
	ian2014, 00:00 Ian2014, 12:00 May2016, 15:26:13	Basin Model: Meteorologic Control Speci	-	x Constant Loss
Show Elements: All El	ements 👻 🗸 Vo	olume Units: 💿 🌆	C-FT Sort	ting: Hydrologic 👻
Hydrologic	Drainage Area	Peak Discharge	Time of Peak	Volume
Element	(MI2)	(CFS)		(IN)
1A	0.4647	255.4	01Jan2014, 12:20	3.7229
2A	0.1519	116.6	01Jan2014, 12:10	3.7206
3A	0.6166	341.3	01Jan2014, 12:20	3.7223
Reach-1a	0.6166	337.1	01Jan2014, 12:25	3.7225
Reach-1b	0.6166	335.5	01Jan2014, 12:30	3.7226
4A	0.3878	177.5	01Jan2014, 12:30	3.5194
5A	0.4725	271.1	01Jan2014, 12:20	3.5686
6A	1.4769	729.3	01Jan2014, 12:25	3.6200
Reach-2	1.4769	727.0	01Jan2014, 12:25	3.6203
7A	0.3240	177.7	01Jan2014, 12:20	3.3605
8A	0.0884	70.5	01Jan2014, 12:10	3.2278
9A	1.8893	924.1	01Jan2014, 12:20	3.5574
Reach-3	1.8893	920.4	01Jan2014, 12:25	3.5573
10A	0.2137	100.3	01Jan2014, 12:20	1.4639
11A	0.1842	100.1	01Jan2014, 12:10	1.4104
Reach-4	2.2872	1060.3	01Jan2014, 12:25	3.1889
12A	2.2872	1068.5	01Jan2014, 12:25	3.1888
13A	0.0237	15.8	01Jan2014, 12:10	1.7228
14A	2.3109	1067.1	01Jan2014, 12:25	3.1738
14R	2.3109	1067.1	01Jan2014, 12:25	3.1738
Outlet	2.3109	1067.1	01Jan2014, 12:25	3.1738

	Global Summar	y Results for Run	"50yr w/ max cl"
--	---------------	-------------------	------------------

- • •

	Project: Kagel_Fin	nal Simulation R	un: 50yr w/ max cl	
Start of Run: 013 End of Run: 023 Compute Time:05M	an2014, 12:00	-	Kagel with Ma Model: 50yr fications:Control 1	x Constant Loss
Show Elements: All Ele	ements 👻 Vo	olume Units: 💿 🌆	🗑 AC-FT Sort	ing: Hydrologic 👻
Hydrologic	Drainage Area	Peak Discharge	Time of Peak	Volume
Element	(MI2)	(CFS)		(IN)
1A	0.4647	363.0	01Jan2014, 12:20	5.6769
2A	0.1519	164.8	01Jan2014, 12:10	5.6447
3A	0.6166	485.0	01Jan2014, 12:20	5.6690
Reach-1a	0.6166	479.8	01Jan2014, 12:25	5.6692
Reach-1b	0.6166	474.9	01Jan2014, 12:30	5.6700
4A	0.3878	253.7	01Jan2014, 12:30	5.4335
5A	0.4725	385.5	01Jan2014, 12:20	5.4529
6A	1.4769	1052.0	01Jan2014, 12:20	5.5384
Reach-2	1.4769	1050.6	01Jan2014, 12:25	5.5386
7A	0.3240	253.0	01Jan2014, 12:20	5.1949
8A	0.0884	100.0	01Jan2014, 12:10	5.0522
9A	1.8893	1354.2	01Jan2014, 12:20	5.4569
Reach-3	1.8893	1341.6	01Jan2014, 12:25	5.4567
10A	0.2137	148.3	01Jan2014, 12:20	2.9410
11A	0.1842	150.7	01Jan2014, 12:10	2.2797
Reach-4	2.2872	1560.4	01Jan2014, 12:25	4.9658
12A	2.2872	1567.8	01Jan2014, 12:25	4.9658
13A	0.0237	23.6	01Jan2014, 12:10	2.6041
14A	2.3109	1571.6	01Jan2014, 12:25	4.9416
14R	2.3109	1571.6	01Jan2014, 12:25	4.9416
Outlet	2.3109	1571.6	01Jan2014, 12:25	4.9416

Global Summary Resu	lts for Run "100yr	w/ max cl"					
	Project: Kagel_Fir	al Simulation Ru	un: 100yr w/max d				
Project, Rager_Final Sindiadon Rdin, 10091 W/ max ci							
Start of Run: 01Jan2014, 00:00 Basin Model: Kagel with Max Constant Loss							
	an2014, 12:00		Model: 100yr				
Compute Time:05N	May2016, 15:28:20	Control Speci	ifications:Control 1				
Show Elements: All Ele	ements 👻 🗸 Vo	olume Units: 💿 ኪ	🗑 AC-FT 🛛 Sort	ing: Hydrologic 👻			
Hydrologic	Drainage Area	Peak Discharge	Time of Peak	Volume			
Element	(MI2)	(CFS)		(IN)			
1A	0.4647	409.2	01Jan2014, 12:20	6.5143			
2A	0.1519	185.4	01Jan2014, 12:10	6.4721			
3A	0.6166	546.6	01Jan2014, 12:20	6.5039			
Reach-1a	0.6166	540.8	01Jan2014, 12:25	6.5078			
Reach-1b	0.6166	532.7	01Jan2014, 12:30	6.5073			
4A	0.3878	286.3	01Jan2014, 12:30	6.2510			
5A	0.4725	434.4	01Jan2014, 12:20	6.2604			
6A	1.4769	1190.8	01Jan2014, 12:20	6.3610			
Reach-2	1.4769	1185.4	01Jan2014, 12:25	6.3611			
7A	0.3240	285.2	01Jan2014, 12:20	5.9825			
8A	0.0884	112.4	01Jan2014, 12:10	5.8198			
9A	1.8893	1536.1	01Jan2014, 12:20	6.2709			
Reach-3	1.8893	1516.2	01Jan2014, 12:25	6.2707			
10A	0.2137	168.8	01Jan2014, 12:20	3.7060			
11A	0.1842	172.5	01Jan2014, 12:10	2.7087			
Reach-4	2.2872	1776.9	01Jan2014, 12:25	5.7442			
12A	2.2872	1779.2	01Jan2014, 12:20	5.7442			
13A	0.0237	26.9	01Jan2014, 12:10	3.0123			
14A	2.3109	1789.9	01Jan2014, 12:25	5.7162			
14R	2.3109	1789.9	01Jan2014, 12:25	5.7162			
Outlet	2.3109	1789.9	01Jan2014, 12:25	5.7162			

	Project: Kagel_Fir	al Simulation Ru	un: 500yr w/ max d	
	an2014,00:00 an2014,12:00 May2016,15:31:28	Basin Model: Meteorologic Control Speci	-	x Constant Loss
Show Elements: All Ele	ements 👻	Volume Units: 🔘 I	IN 🔘 AC-FT Sort	ing: Hydrologic 👻
Hydrologic	Drainage Area	Peak Discharge	Time of Peak	Volume
Element	(MI2)	(CFS)		(IN)
1A	0.4647	515.1	01Jan2014, 12:20	8.4384
2A	0.1519	232.9	01Jan2014, 12:10	8.3663
3A	0.6166	688.0	01Jan2014, 12:20	8.4206
Reach-1a	0.6166	679.4	01Jan2014, 12:25	8.4224
Reach-1b	0.6166	673.7	01Jan2014, 12:25	8.4224
łA	0.3878	361.0	01Jan2014, 12:30	8.1252
5A	0.4725	546.4	01Jan2014, 12:20	8.1047
5A	1.4769	1504.8	01Jan2014, 12:20	8.2427
Reach-2	1.4769	1497.4	01Jan2014, 12:25	8.2429
7A	0.3240	359.3	01Jan2014, 12:20	7.7869
3A	0.0884	141.4	01Jan2014, 12:10	7.6044
A	1.8893	1940.5	01Jan2014, 12:20	8.1348
leach-3	1.8893	1911.8	01Jan2014, 12:25	8.1349
l0A	0.2137	215.5	01Jan2014, 12:20	5.4606
1A	0.1842	222.4	01Jan2014, 12:10	3.8161
Reach-4	2.2872	2252.4	01Jan2014, 12:25	7.5376
12A	2.2872	2265.2	01Jan2014, 12:20	7.5372
13A	0.0237	34.7	01Jan2014, 12:10	4.0405
14A	2.3109	2269.8	01Jan2014, 12:25	7.5018
l4R	2.3109	2269.8	01Jan2014, 12:25	7.5018
Dutlet	2.3109	2269.8	01Jan2014, 12:25	7.5018

E

# APPENDIX B

HEC-SSP RESULTS

Bulletin 17B Frequency Analysis 14 Jan 2015 11:29 AM --------- Input Data ---Analysis Name: 11084000 Description: Data Set Name: ROGERS C-AZUSA CA-FLOW-ANNUAL PEAK DSS File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Kagel.dss DSS Pathname: /ROGERS C/AZUSA CA/FLOW-ANNUAL PEAK/01jan1900/IR-CENTURY/USGS/ Report File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Bulletin17bResults\11084000\11084000.rpt XML File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Bulletin17bResults\11084000\11084000.xml Start Date: End Date: Skew Option: Use Station Skew Regional Skew: -Infinity Regional Skew MSE: -Infinity Plotting Position Type: Median Upper Confidence Level: 0.05 Lower Confidence Level: 0.95 Display ordinate values using 1 digits in fraction part of value --- End of Input Data ---\_\_\_\_\_ << Low Outlier Test >> \_\_\_\_\_ Based on 45 events, 10 percent outlier test deviate K(N) = 2.727Computed low outlier test value = 3.840 low outlier(s) identified below test value of 3.84 \_\_\_\_\_ << High Outlier Test >> \_\_\_\_\_ Based on 45 events, 10 percent outlier test deviate K(N) = 2.727Computed high outlier test value = 9,742.890 high outlier(s) identified above test value of 9,742.89

--- Final Results ---

#### << Plotting Positions >> ROGERS C-AZUSA CA-FLOW-ANNUAL PEAK

	Evel	nts Ana	-	1		ed Events	
Derr	Man	Veen	FLOW		Water		Median
		Year	CFS	Rank 	Year	Cr5 	Plot Pos
10	Mar	1918	332.0	1	1959	2,400.0	1.54
11	Feb	1919	17.0	2	1938	2,070.0	3.74
		1920	206.0	3	1926	1,800.0	5.95
		1921	244.0	4	1943	1,700.0	8.15
		1922	576.0	5	1927	1,200.0	10.35
		1922	130.0	6	1952	867.0	12.56
		1924	28.0	7	1934	825.0	14.76
		1925	555.0	8	1962	700.0	16.96
			1,800.0	9	1935	576.0	19.16
	-	1927	1,200.0	10	1922	576.0	21.37
		1928	64.0	11	1925	555.0	23.57
		1928	62.0	12			
			60.0		1936	520.0	25.77
		1930		13	1944	494.0	27.97
	-	1931	38.0	14	1956	485.0	30.18
		1931	296.0	15	1958	472.0	32.38
		1933	200.0	16	1941	408.0	34.58
		1934	825.0	17	1946	400.0	36.78
	-	1935		18	1918	332.0	38.99
		1936	520.0	19	1954	327.0	41.19
		1936		20	1945	300.0	43.39
02	Mar	1938	2,070.0		1932	296.0	45.59
19	Dec	1938	153.0	22	1947	271.0	47.80
08	Jan	1940	196.0	23	1921	244.0	50.00
04	Mar	1941	408.0	24	1920	206.0	52.20
29	Dec	1941	32.0	25	1933	200.0	54.41
23	Jan	1943	1,700.0	26	1940	196.0	56.61
22	Feb	1944	494.0	27	1937	190.0	58.81
02	Feb	1945	300.0	28	1939	153.0	61.01
23	Dec	1945	400.0	29	1950	132.0	63.22
20	Nov	1946		30	1923		65.42
29	Apr	1948	13.0	31	1953		67.62
	-	1949	22.0	32	1957	90.0	69.82
		1949	132.0	33	1928	64.0	72.03
		1951	10.0	34	1929	62.0	74.23
	-	1952	867.0	35	1961	60.0	76.43
		1952	115.0	36	1930	60.0	78.63
		1954	327.0	37	1960	43.0	80.84
		1955	18.0	38	1931	38.0	83.04
		1956	485.0	39	1942	32.0	85.24
		1957	90.0	40	1924	28.0	87.44
		1958	472.0	40	1949	22.0	89.65
		1958	2,400.0	41	1949	18.0	91.85
		1959	43.0	42			
	-	1960 1961	43.0 60.0		1919	17.0	94.05
				44	1948	13.0	96.26
$\perp \perp$	гер	1962	700.0	45	1951	10.0	98.46

<< Skew Weighting >>

Based on 45 even	nts, mean-square	error of	station	skew =	0.135
Mean-square erro	or of regional s	kew =			-?

<< Frequency Curve >> ROGERS C-AZUSA CA-FLOW-ANNUAL PEAK

	-	Expected Probability CFS	Ì	Percent Chance Exceedance		Confidence 0.05 FLOW, C	0.95
-	7,194.5	8,772.2	-	0.2	-   -	17,008.6	3,827.9
	5,222.5	6,097.5		0.5		11,622.3	2,892.4
	3,974.1	4,504.8		1.0		8,408.3	2,275.4
	2,921.4	3,219.5		2.0		5,846.7	1,734.0
	1,807.5	1,927.8		5.0		3,329.5	1,130.8
	1,157.9	1,207.0		10.0		1,985.6	756.3
	659.0	674.1		20.0		1,043.9	449.7
	207.9	207.9		50.0		298.1	145.6
	59.2	57.6		80.0		86.6	37.6
	29.5	27.9		90.0		45.5	16.9
	16.2	14.8		95.0		26.6	8.4
	5.0	4.1		99.0		9.4	2.1
-			-		-   -		

### << Systematic Statistics >>

ROGERS C-AZUSA CA-FLOW-ANNUAL PEAK

Log Trans			Number of Event	.s		
Mean   Standard Dev   Station Skew   Regional Skew   Weighted Skew   Adopted Skew	2.287 0.624 -0.301  -0.301	-   -             -   -	Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 0 0 0	0	-           

--- End of Analytical Frequency Curve ---

Bulletin 17B Frequency Analysis 14 Jan 2015 11:30 AM \_\_\_\_\_ --- Input Data ---Analysis Name: 11086500 Description: Data Set Name: LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK DSS File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Kagel.dss DSS Pathname: /LITTLE DALTON C/GLENDORA CA/FLOW-ANNUAL PEAK/01jan1900/IR-CENTURY/USGS/ Report File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Bulletin17bResults\11086500\11086500.rpt XML File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Bulletin17bResults\11086500\11086500.xml Start Date: End Date: Skew Option: Use Station Skew Regional Skew: -Infinity Regional Skew MSE: -Infinity Plotting Position Type: Median Upper Confidence Level: 0.05 Lower Confidence Level: 0.95 Display ordinate values using 1 digits in fraction part of value --- End of Input Data ---

--- Preliminary Results ---

Note: Adopted skew equals station skew and preliminary frequency statistics are for the conditional frequency curve because of zero or missing events.

<< Frequency Curve >> LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK

LITTLE	DALTON	C-GLENDORA	CA-FLOW-ANNUAL	PEAK
--------	--------	------------	----------------	------

	Computed Curve FLOW,	Probability	•	Percent Chance xceedance	   	     	Confidence 0.05 FLOW,	0.95
-	3,058.5	4,359.3		0.2	-		10,462.3	1,313.4
	2,063.9	2,715.0		0.5			6,431.0	938.7
	1,480.9	1,843.3	•	1.0			4,270.9	705.9
	1,022.4	1,209.0		2.0			2,710.3	512.6
	577.6	643.9		5.0			1,351.7	311.3
	342.3	366.9		10.0			720.5	195.6
	178.1	184.9		20.0			333.6	107.8
	48.0	48.0		50.0			77.7	29.8
	11.9	11.4		80.0			19.6	6.4
	5.5	5.1		90.0			9.8	2.6
	2.9	2.5		95.0			5.5	1.2
	0.8	0.6		99.0	I		1.9	0.3
-					-			

#### << Conditional Statistics >> LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK

   	Log Transfo FLOW, CFS	rm:	     -   -	Number of Event	s	   
St   St   Re	ean candard Dev cation Skew egional Skew eighted Skew dopted Skew	1.656 0.700 -0.218  -0.218	       	Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 0 1	0         33

<< Conditional Probability Adjusted Ordinates >>

<< Frequency Curve >> LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK

	irve	Expected Probability , CFS	Percent   Chance   Exceedance	Confidence Lim 0.05 FLOW, CFS	
			0.2		
	2,034.9		0.5		
•	, 1,457.8		1.0		İ
	1,004.6		2.0		
	565.3		5.0		
	333.5		10.0		
	172.2		20.0		
	45.0		50.0		
	10.1		80.0		
	4.0		90.0		
	1.4		95.0		
			99.0		

--- End of Preliminary Results ---

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<< Low Outlier Test >>

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Based on 32 events, 10 percent outlier test deviate K(N) = 2.591 Computed low outlier test value = 0.7

0 low outlier(s) identified below test value of 0.7

Based on statistics after 0 zero events and 1 missing events were deleted.

\_\_\_\_\_

<< High Outlier Test >>

\_\_\_\_\_

Based on 32 events, 10 percent outlier test deviate K(N) = 2.591 Computed high outlier test value = 2,942.96

0 high outlier(s) identified above test value of 2,942.96

Note: Statistics and frequency curve were modified using conditional probablity adjustment.

--- Final Results ---

#### << Plotting Positions >> LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK

	Events Ana	-		Ordere		
_		FLOW		Water	FLOW	Median
Day 	Mon Year	CFS	Rank 	Year	CFS	Plot Pos
05	Jan 1939	36.0	1	1962	1,700.0	2.10
07	Jan 1940	63.0	2	1967	325.0	5.09
04	Mar 1941	73.0	3	1961	314.0	8.08
29	Dec 1941	10.0	4	1966	280.0	11.08
23	Jan 1943	182.0	5	1944	198.0	14.07
22	Feb 1944	198.0	6	1943	182.0	17.07
11	Nov 1944	96.0	7	1958	180.0	20.06
21	Dec 1945	111.0	8	1956	180.0	23.05
20	Nov 1946	57.0	9	1963	122.0	26.05
03	Apr 1948	4.0	10	1952	118.0	29.04
04	Mar 1949	1.9	11	1946	111.0	32.04
18	Dec 1949	8.1	12	1945	96.0	35.03
	Jan 1951	5.4	13	1941	73.0	38.02
16	Jan 1952	118.0	14	1959	64.0	41.02
01	Dec 1952	13.0	15	1940	63.0	44.01
25	Jan 1954	58.0	16	1965	62.0	47.01
18	Jan 1955	4.3	17	1954	58.0	50.00
26	Jan 1956	180.0	18	1947	57.0	52.99
13	Jan 1957	12.0	19	1968	49.0	55.99
03	Apr 1958	180.0	20	1939	36.0	58.98
16	Feb 1959	64.0	21	1970	30.0	61.98
08	Feb 1960	2.2	22	1964	28.0	64.97
26	Jan 1961	314.0	23	1971	25.0	67.96
20	Nov 1961	1,700.0	24	1953	13.0	70.96
09	Feb 1963	122.0	25	1957	12.0	73.95
21	Jan 1964	28.0	26	1942	10.0	76.95
09	Apr 1965	62.0	27	1950	8.1	79.94
22	Nov 1965	280.0	28	1951	5.4	82.93
06	Dec 1966	325.0	29	1955	4.3	85.93
	Mar 1968	49.0	30	1948	4.0	88.92
	Jan 1969		31	1960	2.2	91.92
04	Mar 1970	30.0	32	1949	1.9	94.91
21	Dec 1970	25.0	33	1969		97.90

<< Skew Weighting >>

Based on 33 events, mean-	square error	of station	skew = 0.168
Mean-square error of regi	onal skew =		-?

#### << Frequency Curve >> LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK

0.95	Confidence 0.05 FLOW,	Percent Chance Exceedance		Expected Probability CFS	Computed Curve FLOW,
1,316.3	10,390.5	 0.2	6	4,353.6	3,062.4
932.5	6,323.3	0.5	7	2,685.7	2,046.9
696.5	4,167.4	1.0	2	1,810.2	1,457.8
502.2	2,624.3	2.0	6	1,178.6	998.8
302.0	1,295.5	5.0	5	621.5	558.3
188.3	685.1	10.0	4	351.4	328.2
102.8	314.8	20.0	6	175.6	169.3
28.1	72.6	50.0	0	45.0	45.0
6.0	18.2	80.0	6	10.6	11.1
2.4	9.1	90.0	8	4.8	5.2
1.1	5.1	95.0	4	2.4	2.7
0.2	1.7	99.0	6	0.6	0.8

#### << Synthetic Statistics >> LITTLE DALTON C-GLENDORA CA-FLOW-ANNUAL PEAK

Log Transfo   FLOW, CFS			Number of Event	S
Mean   Standard Dev   Station Skew   Regional Skew   Weighted Skew   Adopted Skew	1.630 0.704 -0.202  -0.202	-   -         	Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 0 0 1 33

--- End of Analytical Frequency Curve ---

```
Bulletin 17B Frequency Analysis
 14 Jan 2015 11:32 AM
_____
--- Input Data ---
Analysis Name: 11096500
Description:
Data Set Name: LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK
DSS File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC
SSP\Kagel\Kagel.dss
DSS Pathname: /LITTLE TUJUNGA C/SAN FERNANDO CA/FLOW-ANNUAL
PEAK/01jan1900/IR-CENTURY/USGS/
Report File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC
SSP\Kagel\Bulletin17bResults\11096500\11096500.rpt
XML File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC
SSP\Kagel\Bulletin17bResults\11096500\11096500.xml
Start Date:
End Date:
Skew Option: Use Station Skew
Regional Skew: -Infinity
Regional Skew MSE: -Infinity
Plotting Position Type: Median
Upper Confidence Level: 0.05
Lower Confidence Level: 0.95
Display ordinate values using 1 digits in fraction part of value
--- End of Input Data ---
```

--- Preliminary Results ---

Note: Adopted skew equals station skew and preliminary frequency statistics are for the conditional frequency curve because of zero or missing events.

<< Frequency Curve >>

LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

Computed   Curve   FLOW,	Probability	Percent   Chance   Exceedance	0.05	ce Limits   0.95   , CFS
12,465.5   9,975.4   8,114.9   6,318.0   4,122.1   2,668.7   1,463.3   362.9   63.5   22.0   8.5 	14,198.0 11,139.3 8,941.3 6,850.5 4,380.8 2,781.8 1,499.8 362.9 60.8 20.1 7.3 0.8	1.0   2.0   5.0   10.0   20.0   20.0   50.0   80.0   90.0   95.0		4,977.5   4,144.8   3,316.6   2,260.9   1,523.7   874.4   225.3   34.7   10.4

<< Conditional Statistics >>

LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

Log Transfo FLOW, CFS			Number of Event	S	
Mean	2.453	-   -	Historic Events		0
Standard Dev	0.831	Ì	High Outliers	0	
Station Skew	-0.775	Ì	Low Outliers	0	
Regional Skew			Zero Events	2	
Weighted Skew			Missing Events	0	
Adopted Skew	-0.775		Systematic Events		46

<< Conditional Probability Adjusted Ordinates >>

<< Frequency Curve >> LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

Computed Ex   Curve Pro   FLOW, CE	bability	Percent Chance Exceedance	Confidence Limi   0.05   FLOW, CFS	ts   0.95   
12,337.1		0.2		
9,849.0		0.5		
7,991.2		1.0		
6,204.4		2.0		
4,016.6		5.0		
2,577.4		10.0		
1,393.2		20.0		
325.6		50.0		
45.6		80.0		
10.5		90.0		
0.7		95.0		
		99.0		

--- End of Preliminary Results ---

-----

<< Low Outlier Test >>

Based on 44 events, 10 percent outlier test deviate K(N) = 2.719 Computed low outlier test value = 1.56

1 low outlier(s) identified below test value of 1.56

Based on statistics after 2 zero events and 0 missing events were deleted.

Statistics and frequency curve adjusted for 1 low outlier(s)

<< Conditional Statistics >>

LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

Log Transform:   FLOW, CFS			     Number of Events		
<pre>  Mean   Standard Dev   Station Skew   Regional Skew   Weighted Skew   Adopted Skew</pre>	2.512 0.745 -0.407  -0.775		Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 1 2 0	0 46

<< High Outlier Test >>

\_\_\_\_\_

Based on 43 events, 10 percent outlier test deviate K(N) = 2.71 Computed high outlier test value = 33,988.43

0 high outlier(s) identified above test value of 33,988.43

Note: Statistics and frequency curve were modified using conditional probablity adjustment.

--- Final Results ---

<< Plotting Positions >>

LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

	Events Ana	lyzed	Ordered Events			
		FLOW		Water	FLOW	Median
Day	Mon Year	CFS	Rank	Year	CFS	Plot Pos
31	Dec 1913	4,100.0	   1	1938	8,500.0	1.51
30	Nov 1928	0.0	2	1944	4,220.0	3.66
30	Nov 1929	0.0	3	1914	4,100.0	5.82
04	Feb 1931	30.0	4	1943	3,700.0	7.97
09	Feb 1932	660.0	5	1952	2,110.0	10.13
	Jan 1933	450.0	6	1940	2,090.0	12.28
	Jan 1934	1,360.0	7	1962	1,630.0	14.44
	Dec 1934	89.0	8	1973	1,570.0	16.59
	Feb 1936	653.0	9	1969	1,420.0	18.75
	Feb 1937	964.0	10	1934	1,360.0	20.91
	Mar 1938	8,500.0	11	1941	1,310.0	23.06
	Mar 1939	175.0	12	1966	1,300.0	25.22
	Jan 1940	2,090.0	13	1937	964.0	27.37
	Mar 1940	1,310.0	14	1957	904.0 901.0	29.53
	Dec 1941	198.0	15	1972	762.0	31.68
	Jan 1943	3,700.0	16	1972	660.0	33.84
	Feb 1943	4,220.0	17	1932	653.0	35.99
	Nov 1944	4,220.0	17		569.0	38.15
				1971		
	Mar 1946	156.0	19	1958	559.0	40.30
	Nov 1946	200.0	20	1933	450.0	42.46
	Mar 1948	16.0	21	1956	445.0	44.61
	May 1949	0.9	22	1970	353.0	46.77
	Dec 1949	9.8	23	1961	266.0	48.92
	Jan 1951	13.0	24	1964	256.0	51.08
	Jan 1952	2,110.0	25	1945	244.0	53.23
	Dec 1952	138.0	26	1965	223.0	55.39
	Feb 1954	198.0	27	1947	200.0	57.54
	Jan 1955	35.0	28	1954	198.0	59.70
	Jan 1956	445.0	29	1942	198.0	61.85
	Feb 1957	112.0	30	1939	175.0	64.01
	Apr 1958	559.0	31	1946	156.0	66.16
	Jan 1959	84.0	32	1953	138.0	68.32
	Feb 1960	6.7	33	1968	112.0	
	Nov 1960			1957	112.0	
	Feb 1962			1935	89.0	
10	Feb 1963	52.0	36	1959	84.0	76.94
	Jan 1964	256.0	37	1963	52.0	79.09
09	Apr 1965	223.0	38	1955	35.0	81.25
22	Nov 1965	1,300.0	39	1931	30.0	83.41
06	Dec 1966	901.0	40	1948	16.0	85.56
19	Nov 1967	112.0	41	1951	13.0	87.72
25	Feb 1969	1,420.0	42	1950	9.8	89.87
	Feb 1970	353.0	43		6.7	
	Nov 1970	569.0	44	1949		94.18
	Dec 1971	762.0	45	1930		96.34
		1,570.0	46	1929		98.49

\* Outlier

<< Skew Weighting >>

Based on 46 events, mean-square error of station skew =	0.143
Mean-square error of regional skew =	-?

<< Frequency Curve >> LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

	Curve	Expected Probability , CFS	Percent   Chance   Exceedance	Confidence   0.05   FLOW, C	0.95
	19,338.4 13,698.0 10,155.6 7,210.8 4,182.0 2,495.8 1,281.3 313.6	23,739.0 16,121.7 11,613.6 8,014.5 4,494.5 2,616.2 1,315.2 313.6	0.5         1.0         2.0         5.0         10.0         20.0         50.0	53,109.9         35,308.6         24,803.0         16,580.5         8,772.0         4,827.7         2,259.9         490.0	9,163.1   6,769.0   5,199.4   3,838.7   2,359.5   1,478.9   798.9   202.3
	64.1 26.0 11.8 2.5	61.9 24.2 10.5 1.9	90.0   95.0	102.4   44.5   21.9   5.5	36.7   13.1   5.2   0.8

<< Synthetic Statistics >>

LITTLE TUJUNGA C-SAN FERNANDO CA-FLOW-ANNUAL PEAK

Log Transform:					
FLOW, CFS			Number of Events		
Mean   Standard Dev   Station Skew   Regional Skew   Weighted Skew   Adopted Skew	2.441 0.779 -0.426  -0.426		Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 1 2 0	0         46

--- End of Analytical Frequency Curve ---

Bulletin 17B Frequency Analysis 14 Jan 2015 02:10 PM \_\_\_\_\_ --- Input Data ---Analysis Name: 11098000 Description: Data Set Name: ARROYO SECO-PASADENA CA-FLOW-ANNUAL PEAK DSS File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Kagel.dss DSS Pathname: /ARROYO SECO/PASADENA CA/FLOW-ANNUAL PEAK/01jan1900/IR-CENTURY/USGS/ Report File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Bulletin17bResults\11098000\11098000.rpt XML File Name: C:\Users\wbreakwell\Desktop\Weiwei\Kagel Canyon\HEC SSP\Kagel\Bulletin17bResults\11098000\11098000.xml Start Date: End Date: Skew Option: Use Station Skew Regional Skew: -Infinity Regional Skew MSE: -Infinity Plotting Position Type: Median Upper Confidence Level: 0.05 Lower Confidence Level: 0.95 Display ordinate values using 1 digits in fraction part of value --- End of Input Data ---

--- Preliminary Results ---

Note: Adopted skew equals station skew and preliminary frequency statistics are for the conditional frequency curve because of zero or missing events.

<< Frequency Curve >> ARROYO SECO-PASADENA CA-FLOW-ANNUAL PEAK

	Curve	Expected Probability , CFS	Percent   Chance   Exceedance	Confidence   0.05   FLOW, C	0.95
	16,217.1 12,086.4 9,383.9 7,038.6 4,472.4 2,922.0 1,694.7 545.0 154.6 76.0	12,866.6 9,877.8 7,327.9 4,596.0 2,974.5 1,711.5 545.0 152.6 74.1	0.5         1.0         2.0         5.0         10.0         20.0         80.0         90.0	27,471.4 19,786.7 14,926.7 10,844.9 6,568.4 4,116.6 2,279.4 692.7 200.4 102.6	10,553.5   8,071.8   6,404.4   4,919.5   3,238.1   2,179.3   1,303.9   429.9   115.4   53.4
    -	41.1 12.1	39.4 11.0		58.1   19.1 	27.1   7.0

<< Conditional Statistics >> ARROYO SECO-PASADENA CA-FLOW-ANNUAL PEAK

ARROYO	SECO-PASADENA	CA-FLOW-ANNUAL	PEAK

Log Transf   FLOW, CF			Number of Event	.S
Mean   Standard Dev   Station Skew   Regional Skew   Weighted Skew   Adopted Skew	2.698 0.622 -0.372  -0.372	       	Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 0 0 1 100

<< Conditional Probability Adjusted Ordinates >>

<< Frequency Curve >> ARROYO SECO-PASADENA CA-FLOW-ANNUAL PEAK

Curve	Expected Probability I, CFS	Percent   Chance   Exceedance	Confidence Limits     0.05 0.95     FLOW, CFS
16,167.2	 ?	0.2	
12,043.8		0.5	
9,346.6	5	1.0	
7,006.9	)	2.0	
4,446.9	)	5.0	
2,901.4	l	10.0	
1,678.9	)	20.0	
535.2	2	50.0	
147.5	5	80.0	
69.4	l	90.0	
34.5		95.0	
		99.0	

--- End of Preliminary Results ---

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<< Low Outlier Test >>

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Based on 99 events, 10 percent outlier test deviate K(N) = 3.014 Computed low outlier test value = 6.67

0 low outlier(s) identified below test value of 6.67

Based on statistics after 0 zero events and 1 missing events were deleted.

\_\_\_\_\_

<< High Outlier Test >>

\_\_\_\_\_

Based on 99 events, 10 percent outlier test deviate K(N) = 3.014 Computed high outlier test value = 37,285.21

0 high outlier(s) identified above test value of 37,285.21

Note: Statistics and frequency curve were modified using conditional probablity adjustment.

--- Final Results ---

#### << Plotting Positions >> ARROYO SECO-PASADENA CA-FLOW-ANNUAL PEAK

	Event	ts Anal	-			d Events	
			FLOW		Water	FLOW	Median
Day	Mon Y	Year 	CFS	Rank 	Year	CFS	Plot Pos
	Feb 1		5,800.0	1	1938	8,620.0	0.70
	Feb 1		634.0	2	1969	8,540.0	1.69
	Jan 1		3,150.0	3	1914	5,800.0	2.69
24	Dec 1	1916	760.0	4	1943	5,660.0	3.69
10	Mar 1	1918	570.0	5	1978	5,360.0	4.68
11	Feb 1	1919	92.0	6	2010	4,620.0	5.68
02	Mar 1	1920	450.0	7	1998	4,380.0	6.67
13	Mar 1	1921	650.0	8	1973	3,740.0	7.67
19	Dec 1	1921	2,800.0	9	2005	3,540.0	8.67
13	Dec 1	1922	370.0	10	1966	3,160.0	9.66
26	Mar 1	1924	81.0	11	1916	3,150.0	10.66
04	Apr 1	1925	210.0	12	1980	3,080.0	11.65
	Apr 1		1,450.0	13	1922	2,800.0	12.65
	Feb 1		1,400.0	14	1983	2,640.0	13.65
	Feb 1		298.0	15	2011	2,260.0	14.64
	Apr 1		155.0	16	1935	2,000.0	15.64
	May 1		143.0	17	1944	1,800.0	16.63
	Feb 1		151.0	18	1995	1,730.0	17.63
28	Dec 1	1931	480.0	19	1968	1,720.0	18.63
	Jan 1			20	1993	1,710.0	19.62
	Jan 1		950.0	21	1992	1,710.0	20.62
	Oct 1		2,000.0	22	1967	1,530.0	21.61
	Feb 1		706.0	23	1962	1,500.0	22.61
	Feb 1		640.0	24	1926	1,450.0	23.61
	Mar 1		8,620.0	25	1927	1,400.0	24.60
	Dec 1		375.0	26	1941	1,340.0	25.60
	Jan 1		452.0	27	1971	1,330.0	26.59
	Feb 1		1,340.0	28	1945	1,210.0	27.59
	Dec 1		146.0	29	2006	1,120.0	28.59
	Jan 1		5,660.0	30	1952	1,090.0	29.58
	Feb 1		1,800.0	31	1934	950.0	30.58
	Nov 1		1,210.0	32	1991	921.0	31.57
	Mar 1		680.0	33	2008	892.0	32.57
	Dec 1		600.0	34	1956	815.0	33.57
	Apr 1		45.0	35	1961	769.0	34.56
	Jan 1		35.0	36	1917	760.0	35.56
	Nov 1		150.0	37	1958	715.0	36.55
	Apr 1		12.0	38	1936	706.0	37.55
	Jan 1		1,090.0	39	2004	705.0	38.55
	Dec 1		49.0	40	1946	680.0	39.54
	Jan 1		571.0	41	1970	668.0	40.54
	Apr 1		107.0	42	1921	650.0	41.53
	Jan 1		815.0	43	1937	640.0	42.53
	Feb 1		158.0	44	1915	634.0	43.53
	Apr 1		715.0	45	1981	627.0	44.52
	Feb 1		351.0	46	1982	615.0	45.52
	Jan 1		170.0	47	1947	600.0	46.51
	Nov 1		769.0	48	1976	590.0	47.51
	Feb 1		1,500.0	49	1996	584.0	48.51
			±,000.0	1 12	100	0.10	10.01

09 Fe	b 1963	464.0	50	1954	571.0	49.50
	n 1964	182.0	51	1918	570.0	50.50 j
	r 1965	194.0	52	1997	569.0	51.49
· -	v 1965	3,160.0	53	1975	535.0	52.49
	c 1966	1,530.0	54	2000	509.0	53.49
	v 1967	1,720.0	55	1932	480.0	54.48
	n 1969	8,540.0	56	1963	464.0	55.48
	b 1970	668.0	57	1988	457.0	56.47
	v 1970	1,330.0	58	1940	452.0	57.47
24 De	c 1971	222.0	59	1920	450.0	58.47
11 Fe	b 1973	3,740.0	60	2003	433.0	59.46
08 Ma	r 1974	390.0	61	1974	390.0	60.46
06 Ma	r 1975	535.0	62	1939	375.0	61.45
09 Fel	b 1976	590.0	63	1923	370.0	62.45
09 Ma	y 1977	230.0	64	1959	351.0	63.45
	r 1978	5,360.0	65	2001	348.0	64.44
	b 1979	193.0	66	1928	298.0	65.44
	b 1980	3,080.0	67	2009	270.0	66.43
	n 1981	627.0	68	1977	230.0	67.43
	r 1982	615.0	69	2012	227.0	68.43
	r 1983	2,640.0	70	1972	222.0	69.42
	c 1983	2,040.0	1 71	1984	217.0	70.42
		139.0	71	1984		
	c 1984				213.0	71.41
	n 1986	213.0	73	1925	210.0	72.41
•	n 1987	13.0	74	1965	194.0	73.41
	b 1988	457.0	75	1979	193.0	74.40
	c 1988	155.0	76	1964	182.0	75.40
	b 1990	163.0	77	1960	170.0	76.39
	r 1991	921.0	78	1990	163.0	77.39
	b 1992	1,710.0	-	1957	158.0	78.39
17 Ja:	n 1993	1,710.0	80	1989	155.0	79.38
07 Fe	b 1994	129.0	81	1929	155.0	80.38
10 Ja:	n 1995	1,730.0	82	1931	151.0	81.37
21 Fe	b 1996	584.0	83	1950	150.0	82.37
22 De	c 1996	569.0	84	1942	146.0	83.37
23 Fel	b 1998	4,380.0	85	1930	143.0	84.36
09 Fe	b 1999	62.0	86	1985	139.0	85.36
20 Fe	b 2000	509.0	87	1994	129.0	86.35
	b 2001	348.0		1955	107.0	87.35
	n 2002	41.0	89	1919		88.35
	b 2003	433.0	90	1924		89.34
	b 2004	705.0	91	1999	62.0	90.34
	n 2005	3,540.0	92	1953	49.0	91.33
•	n 2006	1,120.0	93	1948	45.0	92.33
	b 2007	12.0	93	2002	41.0	93.33
	n 2008	892.0	94	1949	41.0 35.0	93.33
	b 2008					94.32   95.32
•		270.0	96	2013	30.0	
	b 2010	4,620.0	97	1987	13.0	96.31
	c 2010	2,260.0	98	2007	12.0	97.31
	r 2012	227.0	99	1951		98.31
I II OC	t 2012	30.0	100	1933		99.30

<< Skew Weighting >>

Based on 100 events, mea	n-square error of station skew = 0.07
Mean-square error of reg	ional skew = -?

<< Frequency Curve >> ARROYO SECO-PASADENA CA-FLOW-ANNUAL PEAK

С	omputed Curve	Expected Probability				Confidence 0.05	
	FLOW	, CFS		Exceedance		FLOW,	CFS
	16,384.0	17,760.4	-	0.2	-   -	27,707.1	10,675.5
	12,110.4	,	•	0.5	İ	19,775.3	8,103.9
	9,346.6	9,845.2	İ	1.0	İ	14,820.4	6,394.8
	6,971.3	7,260.5		2.0		10,701.6	4,886.9
	4,400.5	4,522.4		5.0		6,435.5	3,197.0
	2,863.8	2,915.2		10.0		4,016.7	2,143.8
	1,657.3	1,673.6		20.0		2,219.3	1,279.8
	535.2	535.2		50.0		677.8	423.5
	154.2	152.3		80.0		199.2	115.6
	76.8	74.9	·	90.0		103.3	54.2
	42.1		·	95.0		59.2	28.0
	12.8	11.7		99.0		20.0	7.5

<< Synthetic Statistics >>

ARROYO	SECO-PASADENA	CA-FLOW-ANNUAL	PEAK
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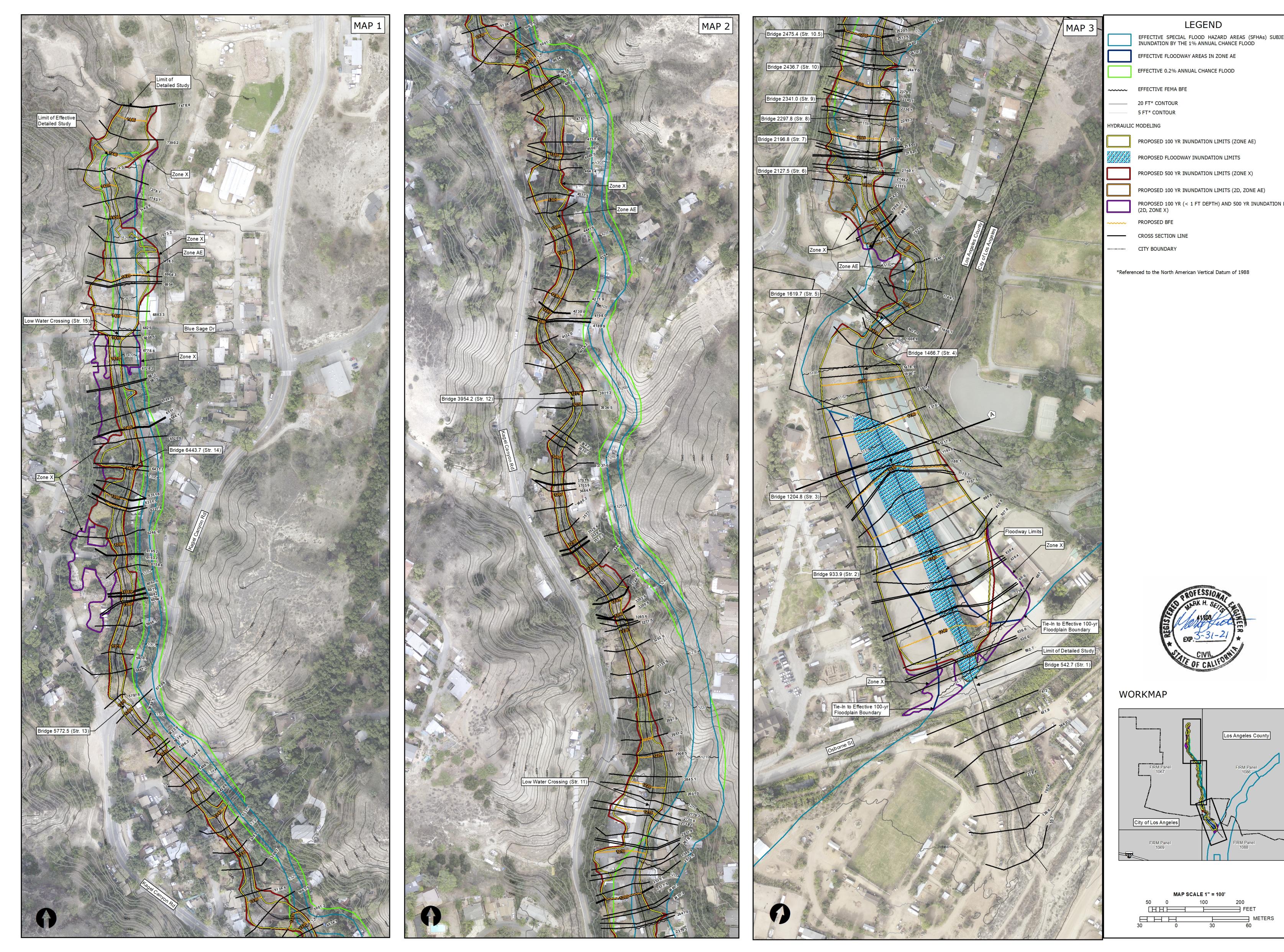
Log Transfo   FLOW, CFS			Number of Event	.S	
Mean   Standard Dev   Station Skew   Regional Skew   Weighted Skew   Adopted Skew	2.694 0.616 -0.341  -0.341	-   -       	Historic Events High Outliers Low Outliers Zero Events Missing Events Systematic Events	0 0 0 1 100	

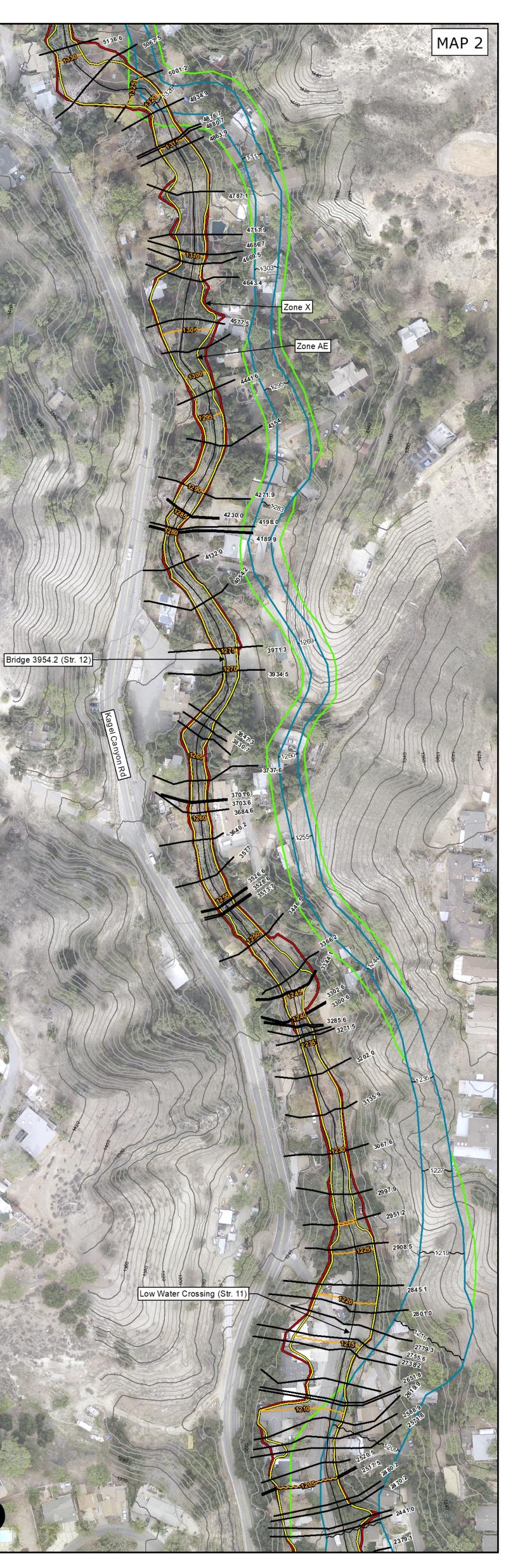
--- End of Analytical Frequency Curve ---

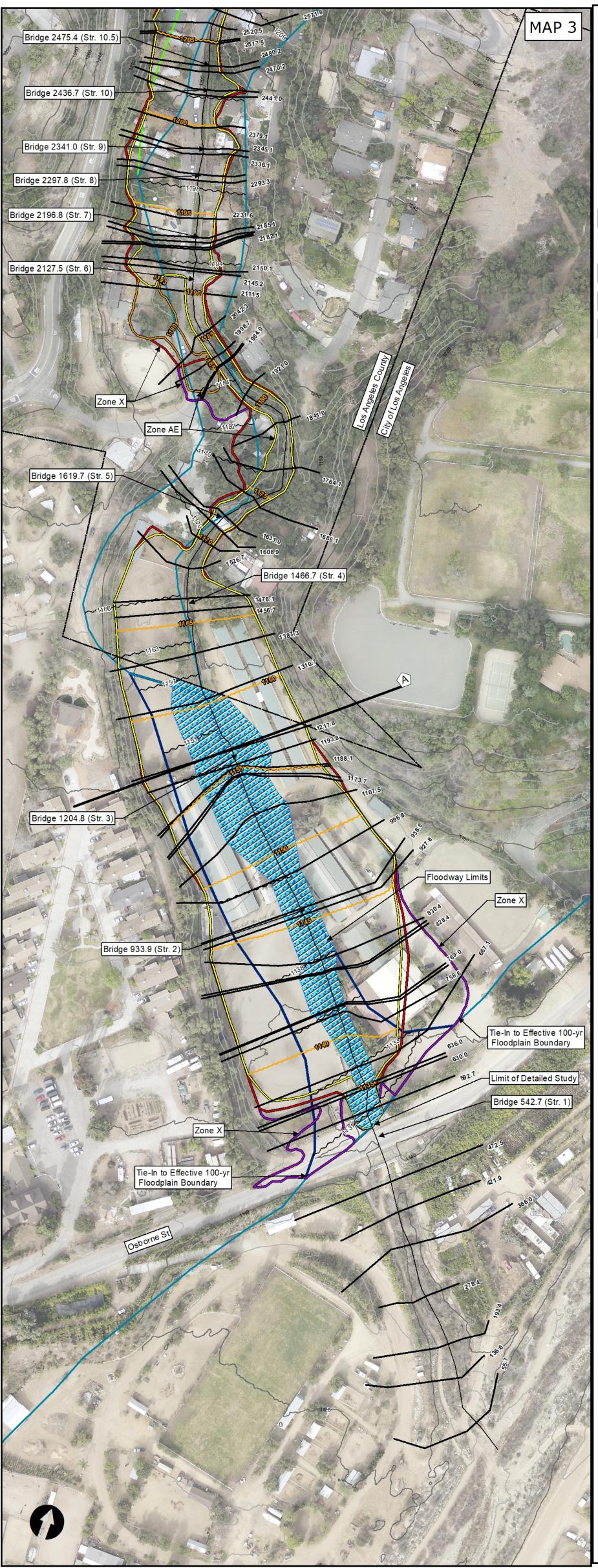
# APPENDIX C

### ELECTRONIC FILES

Appendix D – Kagel Canyon Workmap





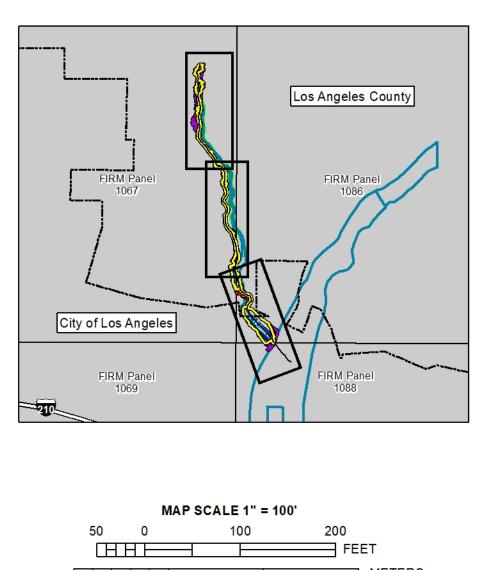


1 >>		LEGEND					
EFFECTIVE SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJE							
	EFFECTIVE FLOODWAY AREAS IN ZONE AE						
EFFECTIVE 0.2% ANNUAL CHANCE FLOOD							
Sec. and	~~~~	EFFECTIVE FEMA BFE					
	<u> </u>	20 FT* CONTOUR					
		5 FT* CONTOUR					
A SALE	HYDRAULIC	MODELING					
		PROPOSED 100 YR INUNDATION LIMITS (ZONE AE)					
No. of Street, or Stre		PROPOSED FLOODWAY INUNDATION LIMITS					
and a second		PROPOSED 500 YR INUNDATION LIMITS (ZONE X)					
AN NOV		PROPOSED 100 YR INUNDATION LIMITS (2D, ZONE AE)					
		PROPOSED 100 YR (< 1 FT DEPTH) AND 500 YR INUNDATION LIMITS (2D, ZONE X)					
	~~~~~	PROPOSED BFE					
	——	CROSS SECTION LINE					
-		CITY BOUNDARY					

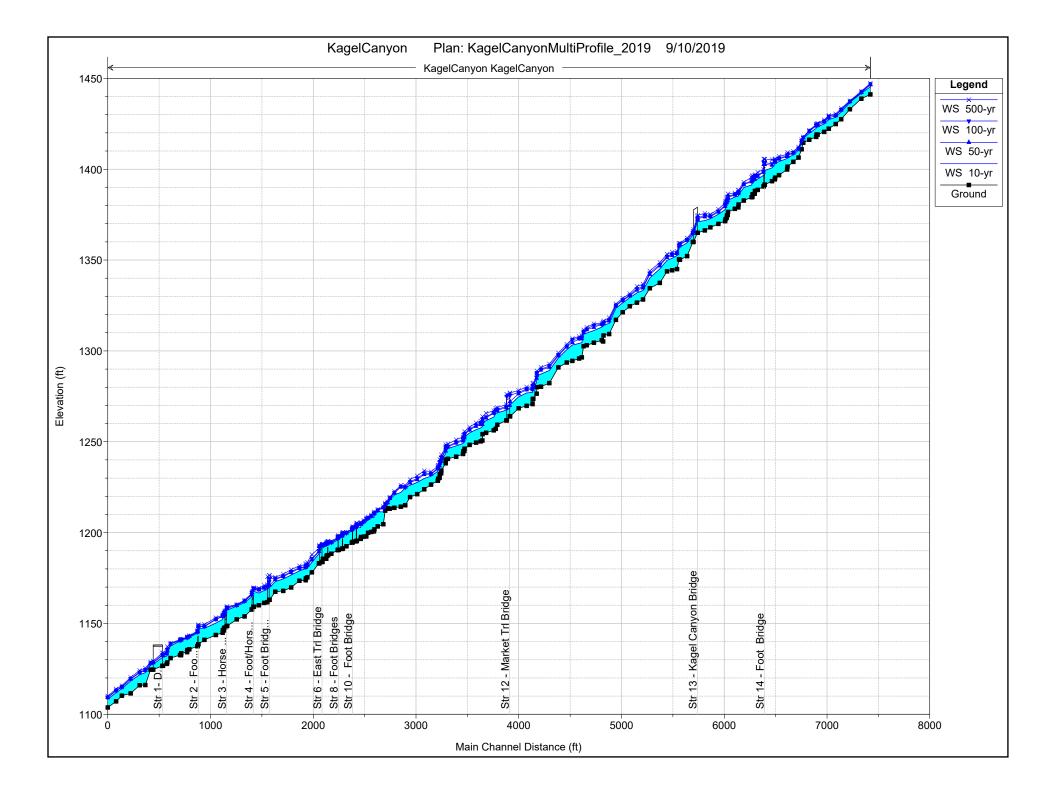
\*Referenced to the North American Vertical Datum of 1988



### WORKMAP



Appendix E – Updated Flood Profile and Encroachment Table



Kagel Canyon Encroachment Summary Table
(Excludes River Stations without Encroachments)

River Sta	Profile	W.S. Elev	Prof Delta WS	Top Wdth Act <sup>1</sup>	Enc WD	Flow Area	Vel Total	Enc Sta L	Enc Sta R
River Sta	FIOINE	(ft)	(ft)	(ft)	(ft)	(sq ft)	(ft/s)	(ft)	(ft)
1387.3	100-yr	1162.43	(10)	78.76	(11)	219.20	8.12	(11)	(11)
1387.3	100-yrEncroached	1162.45	0.02	74.58	74.58	213.20	8.30	91.92	166.50
1307.5	100 yrEnerodened	1102.45	0.02	74.50	74.50	214.45	0.50	51.52	100.50
1310.7	100-yr	1160.12		150.89		306.94	5.80		
1310.7	100-yrEncroached	1160.34	0.22	86.49	86.49	241.01	7.38	121.96	208.45
	,								
1217.8	100-yr	1158.67		127.11		429.35	4.14		
1217.8	100-yrEncroached	1158.68	0.02	121.30	121.30	430.97	4.13	77.69	198.99
1204.8 BR U	100-yr	1158.67		101.11		286.19	6.22		
1204.8 BR U	100-yrEncroached	1158.68	0.02	95.58	121.30	287.34	6.19	77.69	198.99
1204.8 BR D	100-yr	1156.60		101.54		224.40	7.93		
1204.8 BR D	100-yrEncroached	1156.60	0.00	101.54	127.54	224.23	7.93	74.47	202.01
1193.8	100-yr	1156.02		127.54		274.02	6.49		
1193.8	100-yrEncroached	1156.02	0.00	127.54	127.54	274.02	6.49	74.47	202.01
	100			100.10					
1188.1	100-yr	1155.26		126.13	100.07	256.34	6.94	07.50	
1188.1	100-yrEncroached	1155.33	0.07	118.84	120.87	261.63	6.80	97.50	218.37
1100.1	100	1155.01		124.24		256 12	C 05		
1186.1	100-yr 100-yrEncroached	1155.01	0.01	124.34	110 50	256.13	6.95	100.25	210 75
1186.1	100-yrencroached	1155.02	0.01	113.80	118.50	253.16	7.03	100.25	218.75
1175.7	100-yr	1154.39		115.24		239.97	7.41		
1175.7	100-yrEncroached	1154.39	0.00	115.24	116.64	236.20	7.53	115.80	232.44
11/5./	100-yi Liici Oached	1134.33	0.00	100.50	110.04	230.20	7.55	115.80	232.44
1173.7	100-yr	1154.09		99.00		238.53	7.46		
1173.7	100-yrEncroached	1154.10	0.00	97.02	114.47	236.00	7.54	122.36	236.83
1107.5	100-yr	1152.46		94.60		219.26	8.11		
1107.5	, 100-yrEncroached	1152.47	0.01	92.36	123.18	220.19	8.08	112.46	235.64
996.8	100-yr	1148.55		48.90		169.54	10.49		
996.8	100-yrEncroached	1148.83	0.28	51.34	51.34	183.80	9.68	156.04	207.38
938.8	100-yr	1148.61		83.11		366.27	4.86		
938.8	100-yrEncroached	1149.38	0.77	56.40	56.40	347.14	5.12	187.34	243.74
933.9 BR U	100-yr	1148.61		83.11	·	260.21	6.84		
933.9 BR U	100-yrEncroached	1149.38	0.77	56.40	56.40	241.07	7.38	187.34	243.74
0000 000	100	4446.04		472.07		535.00	2 27		
933.9 BR D	100-yr	1146.01	0.00	172.27	FC 40	527.89	3.37	107.00	242.46
933.9 BR D	100-yrEncroached	1146.97	0.96	34.60	56.40	171.45	10.38	187.06	243.46
027.0	100-yr	1145.38		74.21		207.22	0 0 0		
927.8 927.8	100-yr 100-yrEncroached	1145.38 1145.48	0.10	74.31 56.40	56.40	207.32 185.90	8.58 9.57	187.06	243.46
JL1.0	100-yi Elici Uached	1143.48	0.10	30.40	50.40	102.90	9.57	101.00	243.40
849.8	100-yr	1142.93		143.70		259.67	6.85		
849.8	100-yr 100-yrEncroached	1142.95	0.61	60.70	60.70	187.69	9.48	152.46	213.16
043.0	100-yi Liici Oaciieu	1143.34	0.01	00.70	00.70	107.05	9.40	132.40	213.10
830.4	100-yr	1142.40		171.35		291.41	6.10		
030.4	100 yi	1172.40		1,1.32		271.41	0.10	1	1

<b>River Sta</b>	Profile	W.S. Elev	Prof Delta WS	Top Wdth Act <sup>1</sup>	Enc WD	Flow Area	Vel Total	Enc Sta L	Enc Sta R
		(ft)	(ft)	(ft)	(ft)	(sq ft)	(ft/s)	(ft)	(ft)
830.4	100-yrEncroached	1143.17	0.76	60.60	60.60	194.44	9.15	168.50	229.10
828.4	100-yr	1142.27		168.19		283.85	6.27		
828.4	100-yrEncroached	1142.89	0.62	61.29	61.29	190.81	9.32	169.88	231.17
771	100-yr	1141.16		200.05		309.53	5.75		
771	100-yrEncroached	1142.04	0.88	57.00	57.00	203.26	8.75	181.92	238.92
769	100-yr	1141.09		202.07		307.56	5.78		
769	100-yrEncroached	1141.66	0.57	53.14	53.14	182.11	9.77	184.54	237.68
758.6	100-yr	1140.82		206.41		301.84	5.89		
758.6	100-yrEncroached	1141.20	0.38	46.04	46.04	171.29	10.39	177.81	223.85
667.1	100-yr	1138.80		131.90		274.72	6.48		
667.1	100-yrEncroached	1138.81	0.01	34.26	34.26	154.32	11.53	248.05	282.31

### Kagel Canyon Encroachment Summary Table (Excludes River Stations without Encroachments)

1. Note "top width act." represents the top width of the wetted cross section, not including ineffective flow areas. Actual mapped width may vary due to inclusion of ineffective flow areas and blocked obstructions.

Appendix F – Updated Annotated FIRMs

#### NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 11. The horizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202

1315 East–West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the

National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov/. Base map information shown on this FIRM was derived from U.S. Geological Survey

Digital Orthophoto Quadrangles produced at a scale of 1:12,000 from photography dated 1994 or later and from National Geospatial Intelligence Agency imagery produced at a scale of 1:4,000 from photography dated 2003 or later.

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses: and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call1**–877–FEMA MAP**(1–877–336–2627) or visit the FEMA website at http://www.fema.gov/.

> 34°16'52.6 1925000 FT 118 24 22.50

6440000 FT

**FY OF LOS ANGELLES** 

118 24 22.50"

34°18'45.00"

1935000 FT -

1930000 FT -



	LEGEND
	SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD
that has a	al chance flood (100-year flood), also known as the base flood, is the flood 1% chance of being equaled or exceeded in any given year. The Special Area is the area subject to flooding by the 1% annual chance flood. Areas
of Special F	lood Hazard include Zones A, AE, AH, AO, AR, A99, V and VE. The Base in is the water-surface elevation of the 1% annual chance flood.
ZONE A ZONE AE	No Base Flood Elevations determined. Base Flood Elevations determined.
ZONE AE	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood
ZONE AO	Elevations determined. Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain);
704/5 4 5	average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is
	being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations
ZONE V	determined. Coastal flood zone with velocity hazard (wave action); no Base Flood
ZONE VE	Elevations determined. Coastal flood zone with velocity hazard (wave action); Base Flood
	Elevations determined.
	FLOODWAY AREAS IN ZONE AE
kept free of	is the channel of a stream plus any adjacent floodplain areas that must be encroachment so that the 1% annual chance flood can be carried without preases in flood heights.
	OTHER FLOOD AREAS
ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood
	with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance
	flood.
	OTHER AREAS
ZONE D	Areas determined to be outside the 0.2% annual chance floodplain. Areas in which flood hazards are undetermined, but possible.
11111	COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS
2222	OTHERWISE PROTECTED AREAS (OPAs)
CBRS areas a	and OPAs are normally located within or adjacent to Special Flood Hazard Areas.
	<ul> <li>1% annual chance floodplain boundary</li> <li>0.2% annual chance floodplain boundary</li> </ul>
	Floodway boundary Zone D boundary
	CBRS and OPA boundary
********	<ul> <li>Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.</li> </ul>
~~~~ 51	-
(EL 9	elevation in feet*
* Referenced (	to the North American Vertical Datum of 1988 (NAVD 88)
(2)	
97 107'30", :	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
<sup>42</sup> 75 <sup>0</sup>	
60000	00 FT 5000-foot grid ticks: California State Plane coordinate system, V zone (FIPSZONE 0405), Lambert Conformal Conic
DX55	
<b>_</b> M1	<ul> <li>× this FIRM panel)</li> <li>.5 River Mile</li> </ul>
-	
	MAP REPOSITORIES
	Refer to Map Repositories list on Map Index
	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE
	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008
	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008
	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL effective date(S) OF REVISION(S) TO THIS PANEL
Map History f	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL by map revision history prior to countywide mapping, refer to the Community table located in the Flood Insurance Study report for this jurisdiction.
Map History f	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL ey map revision history prior to countywide mapping, refer to the Community table located in the Flood Insurance Study report for this jurisdiction.
Map History f	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL by map revision history prior to countywide mapping, refer to the Community table located in the Flood Insurance Study report for this jurisdiction.
Map History f	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL ex map revision history prior to countywide mapping, refer to the Community table located in the Flood Insurance Study report for this jurisdiction. If flood insurance is available in this community, contact your insurance the National Flood Insurance Program at 1–800–638–6620. MAP SCALE 1" = 500'
Map History f	Befer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         ev map revision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         ev if flood insurance is available in this community, contact your insurance         the National Flood Insurance Program at 1–800–638–6620.         MAP SCALE 1" = 500'         250       0         0       500         1000         1       1         EFEET         MAP SCALE 1" = 500'         250       0         0       500         1000         1       1
Map History f	Refer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         ev map revision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         e if flood insurance is available in this community, contact your insurance         the National Flood Insurance Program at 1–800–638–6620.         MAP SCALE 1'' = 500'         250       0         1       1000         1       1000
Map History f	Befer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         ev map revision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         ev if flood insurance is available in this community, contact your insurance         the National Flood Insurance Program at 1–800–638–6620.         MAP SCALE 1" = 500'         250       0         0       500         1000         1       1         EFFECTIVE DATE(S)
Map History f	Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL expression history prior to countywide mapping, refer to the Community table located in the Flood Insurance Study report for this jurisdiction. If flood insurance is available in this community, contact your insurance the National Flood Insurance Program at 1–800–638–6620. MAP SCALE 1" = 500' 250 0 1000 150 300 METERS
Map History f	Refer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         by map revision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         a if flood insurance is available in this community, contact your insurance         the National Flood Insurance Program at 1–800–638–6620.         MAP SCALE 1" = 500'         250       0         150       0         150       0         150       0         PANEL 1067F
Map History f	Befer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         And prevision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         If flood insurance is available in this community, contact your insurance         MAP SCALE 1" = 500'         250         O         150         O         DATE 1067F         FIREM
Map History f	Refer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         by map revision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         a if flood insurance is available in this community, contact your insurance         the National Flood Insurance Program at 1–800–638–6620.         MAP SCALE 1" = 500'         250       0         150       0         150       0         150       0         PANEL 1067F
Map History f	Befer to Map Repositories list on Map Index         EFFECTIVE DATE OF COUNTYWIDE         FLOOD INSURANCE RATE MAP         September 26, 2008         EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL         And prevision history prior to countywide mapping, refer to the Community         table located in the Flood Insurance Study report for this jurisdiction.         If flood insurance is available in this community, contact your insurance         MAP SCALE 1" = 500'         250         O         150         O         DATE 1067F         FIREM
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#### NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 11. The horizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East–West Highway

Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov/.

Base map information shown on this FIRM was derived from U.S. Geological Survey Digital Orthophoto Quadrangles produced at a scale of 1:12,000 from photography dated 1994 or later and from National Geospatial Intelligence Agency imagery produced at a scale of 1:4,000 from photography dated 2003 or later.

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

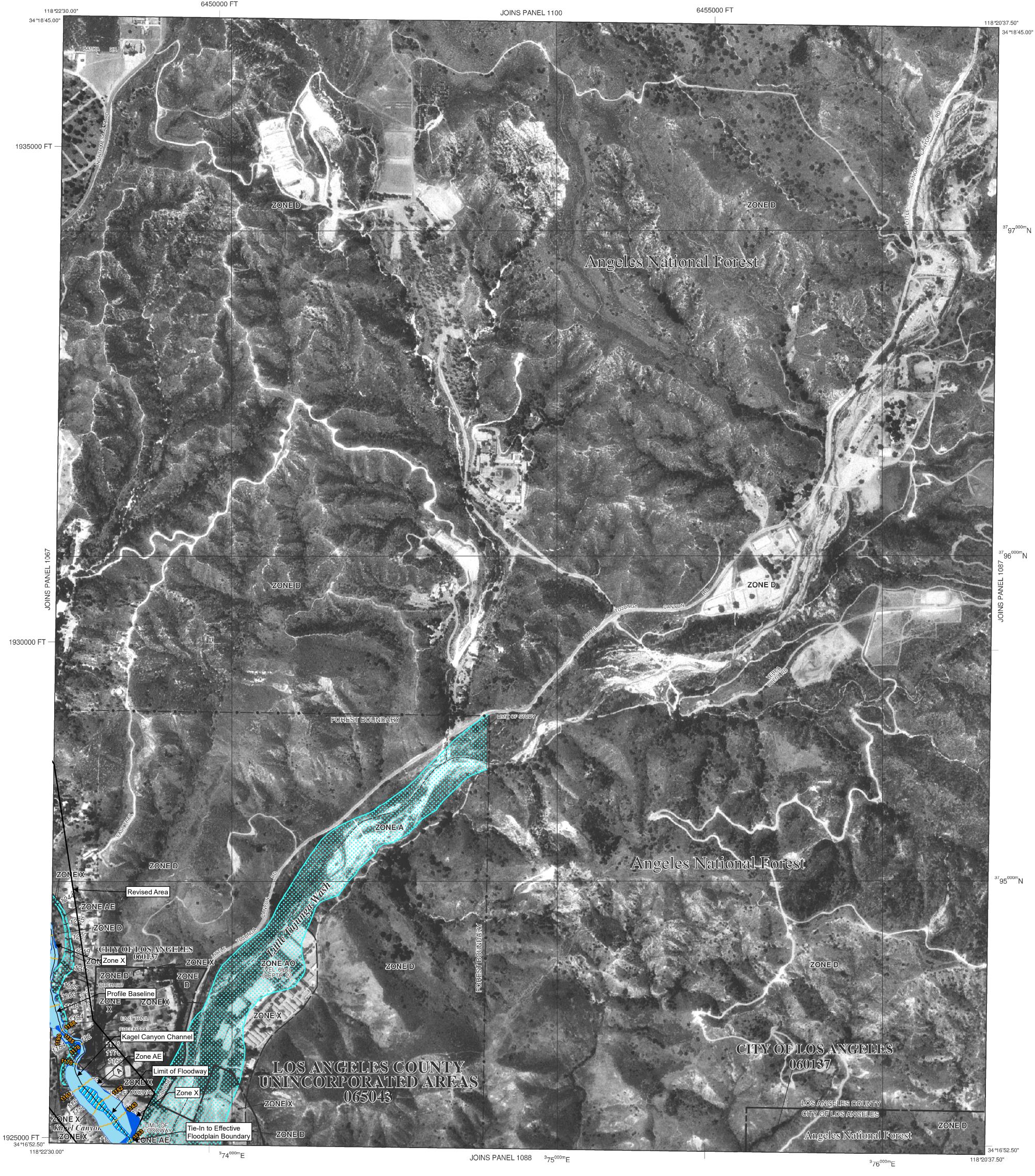
Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, *a Flood Insurance Study report*, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call**1–877–FEMA MAP**(1–877–336–2627) or visit the FEMA website at http://www.fema.gov/.

6450000 FT



	SPECIAL F	LEGEND LOOD HAZARD AREAS (SFHAS) SUBJECT TO
The 1%	INUNDATIC	DN BY THE 1% ANNUAL CHANCE FLOOD
that has a Flood Hazard of Special F	1% chance of d Area is the an flood Hazard ir	being equaled or exceeded in any given year. The Special rea subject to flooding by the 1% annual chance flood. Areas include Zones A, AE, AH, AO, AR, A99, V and VE. The Base rface elevation of the 1% annual chance flood.
ZONE A	No Base Flood	Elevations determined.
ZONE AE ZONE AH	Flood depths	vations determined. s of 1 to 3 feet (usually areas of ponding); Base Flood
ZONE AO		ermined. s of 1 to 3 feet (usually sheet flow on sloping terrain); ns determined. For areas of alluvial fan flooding, velocities
ZONE AR	also determine	d.
	chance flood decertified. Z	I by a flood control system that was subsequently one AR indicates that the former flood control system is d to provide protection from the 1% annual chance or
ZONE A99		protected from 1% annual chance flood by a Federal on system under construction; no Base Flood Elevations
ZONE V		zone with velocity hazard (wave action); no Base Flood
ZONE VE		zone with velocity hazard (wave action); Base Flood
	Elevations dete	AREAS IN ZONE AE
The floodway kept free of	is the channel	of a stream plus any adjacent floodplain areas that must be to that the 1% annual chance flood can be carried without
	OTHER FLO	
ZONE X	with average	2% annual chance flood; areas of 1% annual chance flood depths of less than 1 foot or with drainage areas less than ile; and areas protected by levees from 1% annual chance
	OTHER ARE	AS
ZONE X	Areas determi	ned to be outside the 0.2% annual chance floodplain.
ZONE D		h flood hazards are undetermined, but possible.
(1111)	COASTAL E	BARRIER RESOURCES SYSTEM (CBRS) AREAS
CBRS areas		E PROTECTED AREAS (OPAs) ormally located within or adjacent to Special Flood Hazard Areas. 1% annual chance floodplain boundary
		0.2% annual chance floodplain boundary Floodway boundary
		Zone D boundary
		CBRS and OPA boundary - Boundary dividing Special Flood Hazard Areas of different Base Flood Flooring flood dopths or flood velocities
~~~~ 51	13 ~~~~	Base Flood Elevations, flood depths or flood velocities. Base Flood Elevation line and value; elevation in feet*
	987)	Base Flood Elevation value where uniform within zone; elevation in feet*
* Referenced	to the North Ame	elevation in feet* erican Vertical Datum of 1988 (NAVD 88)
                                                                         	(A)	Cross section line
(23)		Transect line Geographic coordinates referenced to the North American
97°07'30",	32°22'30"	\$ 1
40 5		Datum of 1983 (NAD 83)
	<sup>000m</sup> N	1000-meter Universal Transverse Mercator grid values, zone 11
		1000-meter Universal Transverse Mercator grid values, zone 11
60000	<sup>000m</sup> N	1000-meter Universal Transverse Mercator grid values, zone 11 5000-foot grid ticks: California system, V zone (FIPSZONE 0405), State Plane coordinate Lambert Conformal Conic Bench mark (see explanation in Notes to Users section of
60000	<sup>000m</sup> N 000 FT 510 <sub>×</sub>	1000-meter Universal Transverse Mercator grid values, zone 11 5000-foot grid ticks: California State Plane coordinate system, V zone (FIPSZONE 0405), Lambert Conformal Conic
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# NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 11. The horizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202

1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov/.

**Base map** information shown on this FIRM was derived from U.S. Geological Survey Digital Orthophoto Quadrangles produced at a scale of 1:12,000 from photography dated 1994 or later and from National Geospatial Intelligence Agency imagery produced at a scale of 1:4,000 from photography dated 2003 or later.

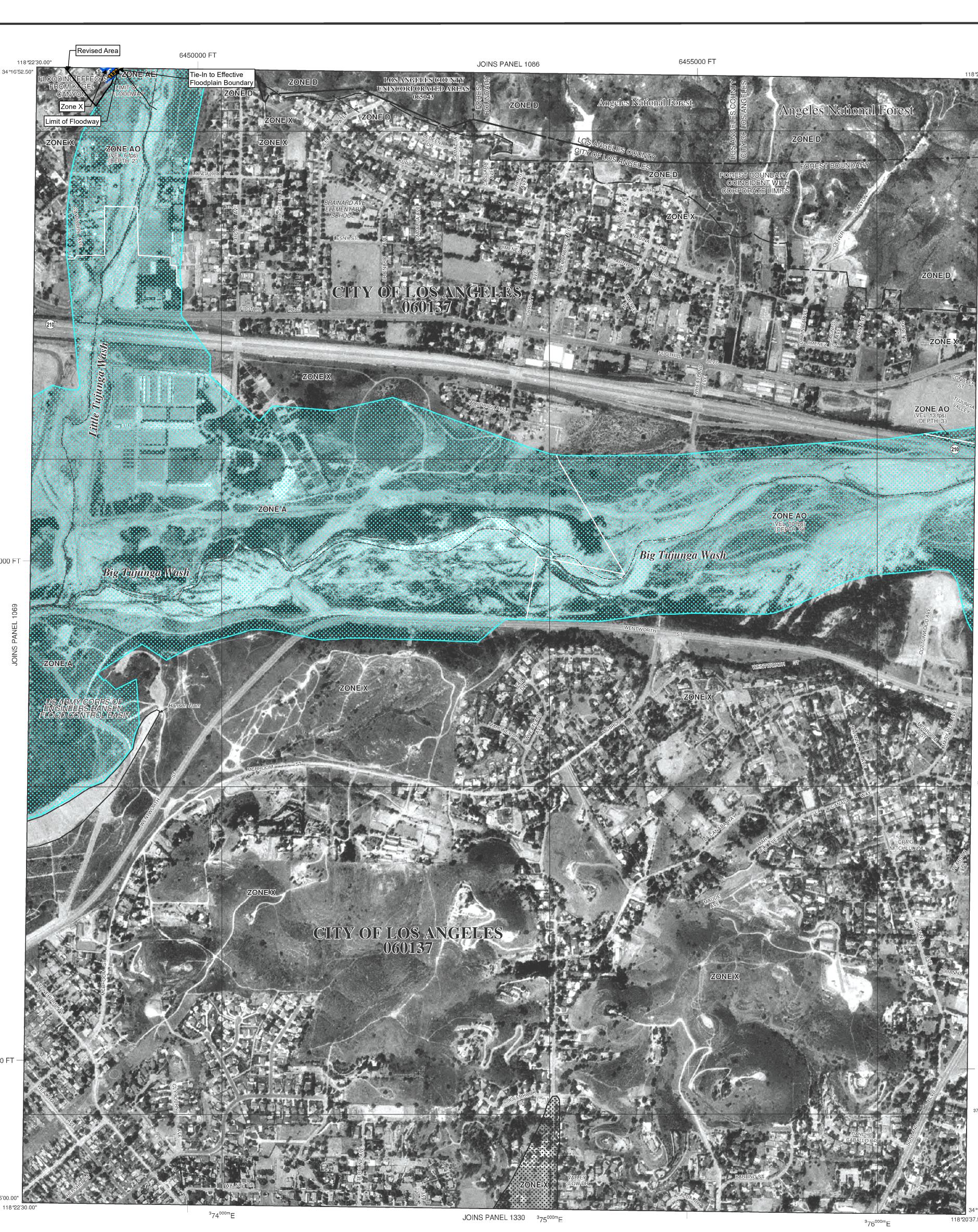
This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call**1–877–FEMA MAP**(1–877–336–2627) or visit the FEMA website at http://www.fema.gov/.



1915000 FT

34 °15'00

1920000 FT

118°20'37.50" 34°16'52.50"	
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118°20'37.50"

